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USACE / NAVFAC / AFCEA UFGS-03931 (May 2004)  
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Preparing Activity: USACE MasterFormat™ 2004 - 03 01 32

UNIFIED FACILITIES GUIDE SPECIFICATION

References are in agreement with UMRL dated 23 June 2005

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SECTION 03931

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05/04

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USACE / NAVFAC / AFCEA UFGS-03931 (May 2004)  
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Preparing Activity: USACE MasterFormat™ 2004 - 03 01 32

## UNIFIED FACILITIES GUIDE SPECIFICATION

References are in agreement with UMRL dated 23 June 2005

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### SECTION 03931

#### CONCRETE REHABILITATION FOR CIVIL WORKS 05/04

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NOTE: This guide specification covers the requirements for rehabilitation of concrete for Civil Works type structures.

This guide specification was prepared to be compatible with the guidance given in EM 1110-2-2000, EM 1110-2-2002, ACI 224.1R, UFC 3-270-03 and UFC 3-270-04.

Comments and suggestions on this guide specification are welcome and should be directed to the technical proponent of the specification. A listing of technical proponents, including their organization designation and telephone number, is on the Internet.

Recommended changes to a UFGS should be submitted as a Criteria Change Request (CCR).

Use of electronic communication is encouraged.

Brackets are used in the text to indicate designer choices or locations where text must be supplied by the designer. Use of electronic communication is encouraged.

This guide specification includes tailoring options for controlled blasting, crack repair, joint resealing, use of precast concrete, overlay, and refacing. Selection or deselection of a tailoring option will include or exclude that option in the section, but editing the resulting section to fit the project is still required.

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NOTE: The following information should be shown on the project drawings:

1. Location and extent of spalled, cracked or damaged concrete to be repaired with epoxy;

2. Location and extent of cracked concrete or saw kerfs to be filled with epoxy mortar;
3. Location of dowels to be installed and cracks to be repaired with non-pressure grout; and
4. Location and sizes of cracks to be filled by pressure grouting.

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NOTE: This guide specification is intended to be used in conjunction with other guide specifications for cement-based concrete materials. Normally, the applicable formwork, joint construction, reinforcing and structural concrete sections should be included. For smaller projects, the specification section 03307 CONCRETE FOR MINOR STRUCTURES should be included. Determination of appropriate guide specifications should be made in accordance with EM 1110-2-2000, Chapter 5. Where only small amounts of cement-based concrete are involved, the applicable portions of the appropriate guide specifications may be incorporated into this section. The following guide specifications are referenced in this specification:

01270A MEASUREMENT AND PAYMENT  
 01572 CONSTRUCTION AND DEMOLITION WASTE  
 MANAGEMENT  
 01355A ENVIRONMENTAL PROTECTION  
 02760A FIELD MOLDED SEALANTS FOR SEALING JOINTS  
 IN RIGID PAVEMENTS  
 03101A FORMWORK FOR CONCRETE  
 03151A EXPANSION, CONTRACTION AND CONSTRUCTION  
 JOINTS IN CONCRETE FOR CIVIL WORKS  
 03201 STEEL BARS AND WELDED WIRE FABRIC FOR  
 CONCRETE REINFORCEMENT FOR CIVIL WORKS  
 03300A CAST-IN-PLACE STRUCTURAL CONCRETE  
 03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR  
 CIVIL WORKS  
 03307 CONCRETE FOR MINOR STRUCTURES  
 03371 SHOTCRETE  
 03372 PREPLACED-AGGREGATE CONCRETE  
 03415A PRECAST-PRESTRESSED CONCRETE

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## PART 1 GENERAL

### 1.1 REFERENCES

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NOTE: This paragraph is used to list the publications cited in the text of the guide specification. The publications are referred to in the text by basic designation only and listed in this paragraph by organization, designation, date, and title.

Use the Reference Wizard's Check Reference feature when you add a RID outside of the Section's Reference Article to automatically place the reference in the Reference Article. Also use the Reference Wizard's Check Reference feature to update the issue dates.

References not used in the text will automatically be deleted from this section of the project specification when you choose to reconcile references in the publish print process.

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The publications listed below form a part of this specification to the extent referenced. The publications are referred to in the text by basic designation only.

ACI INTERNATIONAL (ACI)

ACI 117	(1990; R 2002) Standard Tolerances for Concrete Construction and Materials & Commentary
ACI 211.1	(1991; R 2002) Standard Practice for Selecting Proportions for Normal, Heavyweight, and Mass Concrete
ACI 318/318R	(2002) Building Code Requirements for Structural Concrete and Commentary
ACI 504R	(1990, R 1997) Guide to Sealing Joints in Concrete Structures
ACI 548.4	(1993; R 1998) Latex-Modified Concrete (LMC) Overlays
ACI SP-66	(2004) ACI Detailing Manual

AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO)

AASHTO M 288	(2000) Geotextile Specifications for Highway Applications
AASHTO PP34	(2000) Standard Practice for Estimating the Cracking Tendency of Concrete - 2000 Edition

ASTM INTERNATIONAL (ASTM)

ASTM A 36/A 36M	(2004) Carbon Structural Steel
ASTM A 615/A 615M	(2004b) Deformed and Plain Billet-Steel Bars for Concrete Reinforcement
ASTM A 722/A 722M	(1998; R 2003) Uncoated High-Strength Steel Bar for Prestressing Concrete
ASTM A 775/A 775M	(2001) Epoxy-Coated Reinforcing Steel Bars

ASTM A 820	(2004) Steel Fibers for Fiber-Reinforced Concrete
ASTM A 996/A 996M	(2004) Rail-Steel and Axle-Steel Deformed Bars or Concrete Reinforcement
ASTM C 1012	(2004) Length Change of Hydraulic-Cement Mortars Exposed to a Sulfate Solution
ASTM C 1017/C 1017M	(2003) Chemical Admixtures for Use in Producing Flowing Concrete
ASTM C 1059	(1999) Latex Agents for Bonding Fresh to Hardened Concrete
ASTM C 1077	(2003a) Laboratories Testing Concrete and Concrete Aggregates for Use in Construction and Criteria for Laboratory Evaluation
ASTM C 109/C 109M	(2002) Compressive Strength of Hydraulic Cement Mortars (Using 2-in. [50-mm] Cube Specimens)
ASTM C 1116	(2003) Fiber-Reinforced Concrete and Shotcrete
ASTM C 117	(2004) Materials Finer Than 75 micrometer (No. 200) Sieve in Mineral Aggregates by Washing
ASTM C 136	(2004) Sieve Analysis of Fine and Coarse Aggregates
ASTM C 143/C 143M	(2003) Slump of Hydraulic Cement Concrete
ASTM C 1438	(1999e1) Latex and Powder Polymer Modifiers for Hydraulic Cement Concrete and Mortar
ASTM C 144	(2003) Aggregate for Masonry Mortar
ASTM C 150	(2004a) Portland Cement
ASTM C 157/C 157M	(2004) Length Change of Hardened Hydraulic-Cement Mortar and Concrete
ASTM C 172	(2004) Sampling Freshly Mixed Concrete
ASTM C 192/C 192M	(2002) Making and Curing Concrete Test Specimens in the Laboratory
ASTM C 231	(2004) Air Content of Freshly Mixed Concrete by the Pressure Method
ASTM C 273	(2000e1) Shear Properties of Sandwich Core Materials

ASTM C 31/C 31M	(2003a) Making and Curing Concrete Test Specimens in the Field
ASTM C 33	(2003) Concrete Aggregates
ASTM C 39/C 39M	(2004) Compressive Strength of Cylindrical Concrete Specimens
ASTM C 469	(2002) Static Modulus of Elasticity and Poisson's Ratio of Concrete in Compression
ASTM C 494/C 494M	(2004) Chemical Admixtures for Concrete
ASTM C 596	(2001) Drying Shrinkage of Mortar Containing Hydraulic Cement
ASTM C 666/C 666M	(2003) Resistance of Concrete to Rapid Freezing and Thawing
ASTM C 881/C 881M	(2002) Epoxy-Resin-Base Bonding Systems for Concrete
ASTM C 882	(1999) Bond Strength of Epoxy-Resin Systems Used with Concrete by Slant Shear
ASTM C 928	(2000) Packaged, Dry, Rapid-Hardening Cementitious Materials for Concrete Repairs
ASTM C 937	(2002) Grout Fluidifier for Preplaced-Aggregate Concrete
ASTM C 940	(1998a; R 2003) Expansion and Bleeding of Freshly Mixed Grouts for Preplaced-Aggregate Concrete in the Laboratory
ASTM D 1623	(2003) Tensile and Tensile Adhesion Properties of Rigid Cellular Plastics
ASTM D 2103	(2003) Polyethylene Film and Sheeting
ASTM D 2240	(2004) Rubber Property - Durometer Hardness
ASTM D 226	(1997a) Asphalt-Saturated Organic Felt Used in Roofing and Waterproofing
ASTM D 2471	(1999) Gel Time and Peak Exothermic Temperature of Reacting Thermosetting Resins
ASTM D 2822	(1991; R 1997e1) Asphalt Roof Cement
ASTM D 323	(1999a) Vapor Pressure of Petroleum Products (Reid Method)
ASTM D 3418	(2003) Transition Temperatures of Polymers by Differential Scanning Calorimetry
ASTM D 395	(2003) Rubber Property - Compression Set

ASTM D 4016	(2002) Viscosity of Chemical Grouts by Brook field Viscometer (Laboratory Method)
ASTM D 4101	(2004a) Polypropylene Injection and Extrusion Materials
ASTM D 412	(1998a; R 2002e1) Vulcanized Rubber and Thermoplastic Elastomers - Tension
ASTM D 450	(1996; R 2000e1) Coal-Tar Pitch Used in Roofing, Dampproofing, and Waterproofing
ASTM D 471	(1998e1) Rubber Property - Effect of Liquids
ASTM D 4869	(2004) Asphalt-Saturated Organic Felt Underlayment Used in Steep Slope Roofing
ASTM D 572	(2004) Rubber Deterioration by Heat and Oxygen
ASTM D 75	(2003) Sampling Aggregates
ASTM E 488	(1996; R 2003) Strength of Anchors in Concrete and Masonry Elements

CONCRETE SAWING AND DRILLING ASSOCIATION (CSDA)

CSDA-W-104	(June 1, 1998) Track Mounted Wall Sawing
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POST-TENSIONING INSTITUTE (PTI)

PTI A	(Nov 1990) Post-Tensioning Manual
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PRECAST/PRESTRESSED CONCRETE INSTITUTE (PCI)

PCI MNL-116	(1999) Quality Control for Plants and Production of Structural Precast Concrete Products
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U.S. ARMY CORPS OF ENGINEERS (USACE)

COE CRD-C 164	(1992) Standard Test Method for Direct Tensile Strength of Cylindrical Concrete or Mortar Specimens
COE CRD-C 39	(1981) Test Method for Coefficient of Linear Thermal Expansion of Concrete
COE CRD-C 400	(1963) Requirements for Water for Use in Mixing or Curing Concrete
EM 385-1-1	(2003) Safety -- Safety and Health Requirements

U.S. GENERAL SERVICES ADMINISTRATION (GSA)

CID A-A-1922	(Rev A; Notice 1) Shield, Expansion
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	(Caulking Anchors, Single Lead)
CID A-A-55614	(Basic; Notice 1) Shield, Expansion (Non-Drilling Expansion Anchors)
FS A-A-1923	(Rev A; Notice 1) Shield, Expansion (Lag, Machine and Externally threaded Wedge Bolt Anchors)

## 1.2 DEFINITIONS

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**NOTE: See ACI 201.1 for definitions of various types  
of concrete deterioration.**  
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### 1.2.1 Concrete

A mixture of binder material, water, and fine and coarse aggregate, with or without admixtures.

### 1.2.2 Mortar

A mixture of binder material, water, and fine aggregate, with or without admixtures.

### 1.2.3 Grout

A mixture of binder material and water, with or without a filler.

### 1.2.4 Cement-Based Material

A material consisting of portland cement and/or other cementitious materials as a binder and aggregate. As used in this specification, cement-based materials do not include materials with polymer modifiers.

### 1.2.5 Polymer-Modifier

A polymer used to modify the properties of a cement-based concrete or mortar. The polymers commonly associated with concrete rehabilitation are styrene-butadiene and acrylic latex.

### 1.2.6 Polymer-Modified Material

A combination of polymer, portland cement and/or other cementitious materials, and fine and/or coarse aggregate. Polymer-modified concrete is normally placed to a thickness of 25 mm (1 inch) 1 inch or greater. Polymer-modified mortar is normally placed to a thickness of less than 25 mm (1 inch) 1 inch.

### 1.2.7 Epoxy Resin Binder

A two-component epoxy resin binder system in low and medium viscosities used by itself as a primer or for producing epoxy concrete or mortars when mixed with aggregate.

### 1.2.8 Epoxy Concrete

A combination of epoxy resin and fine and coarse aggregate.

#### 1.2.9 Epoxy Mortar

A combination of epoxy resin, a mineral filler, and fine aggregate.

#### 1.2.10 Non-Pressure Epoxy Grout

A combination of epoxy resin binder, a mineral filler and a thixotropic agent.

#### 1.2.11 Epoxy Injection Adhesive

A low viscosity epoxy resin system pumped under pressure into cracks.

#### 1.2.12 Rapid-Hardening Repair Material

A combination of rapid-setting cement and aggregate(s) that can develop sufficient compressive strength at an early age to permit intended use.

#### 1.2.13 Non-Structural Cracks

Cracks that do not affect the load-carrying capacity of the structure.

#### 1.2.14 Structural Cracks

Cracks that affect the load-carrying capacity of the structure.

### 1.3 UNIT PRICES

\*\*\*\*\*  
NOTE: If Section 01270A MEASUREMENT AND PAYMENT is included in the project specifications, this paragraph title (UNIT PRICES) should be deleted from this section and the remaining appropriately edited subparagraphs below should be inserted into Section 01270A. For additional guidance on payment items, see International Concrete Repair Institute Guidelines No. 03735 "Guide for Methods of Measurement and Contract Types for Concrete Repair Work".

\*\*\*\*\*  
NOTE: For small projects where other relevant guide specifications are not included in the project specification, insert applicable measurement and payment paragraphs either here or in Section 01270A.  
\*\*\*\*\*

#### 1.3.1 Concrete Removal

\*\*\*\*\*  
NOTE: Repeat this bid item and its respective subparagraphs for each bid item of concrete removal, renumbering the bid items appropriately. Unit price bid items should be inserted in paragraph UNIT PRICE BID ITEMS of Section 01270A MEASUREMENT AND PAYMENT. Where concrete removal depth is uniform across an area, measurement and payment may be made on an area

basis, in which case the "ALTERNATE" paragraph may be used.

\*\*\*\*\*

#### 1.3.1.1 Payment

Payment will be made for costs associated with concrete removal, which includes preparatory work, removal of concrete and embedded items, and disposal of debris. Payment will be made at the contract price per cubic meter yard of concrete removed. No payment will be made for concrete removal, as such, that is in connection with items for which payment is made as a lump sum.

#### 1.3.1.2 Measurement

Concrete removal will be measured for payment based upon the actual volume of concrete removed within the pay lines as indicated on the drawings. Measurement will be made in accordance with paragraph FIELD MEASUREMENT. No measurement will be made of concrete removal outside the pay lines shown which is incidental to the indicated removal unless such removal is specifically authorized by the Contracting Officer. No deductions will be made for rounded or beveled edges or for voids or embedded items that are either less than 0.15 cubic meters (five cubic feet) five cubic feet in volume or 0.10 square meter (one square foot) one square foot in cross section.

#### 1.3.1.3 Unit of Measure

Unit of measure: cubic meters yards.

#### 1.3.2 Concrete Removal (Alternate)

\*\*\*\*\*

NOTE: Repeat this bid item and its respective subparagraphs for each bid item of concrete removal, renumbering the bid items appropriately. Unit price bid items should be inserted in paragraph UNIT PRICE BID ITEMS of Section 01270A MEASUREMENT AND PAYMENT. Where concrete removal depth is not uniform across an area, measurement and payment should be made on a volume basis basis, in which case the previous paragraph should be used.

\*\*\*\*\*

#### 1.3.2.1 Payment

Payment will be made for costs associated with concrete removal, which includes preparatory work, removal of concrete and embedded items, and disposal of debris. Payment will be made at the contract price per square meter yard of concrete removed. No payment will be made for concrete removal, as such, that is in connection with items for which payment is made as a lump sum.

#### 1.3.2.2 Measurement

Concrete removal will be measured for payment based upon the actual area of concrete removed within the pay lines as indicated on the drawings. No measurement will be made of concrete removal outside the pay lines shown which is incidental to the indicated removal.

#### 1.3.2.3 Unit of Measure

Unit of measure: square meters yards.

#### 1.3.3 Drilling and Grouting Dowels and Anchors

##### 1.3.3.1 Payment

Payment will be made for costs associated with drilling holes and grouting dowels and anchors for anchoring new concrete and for repairs to existing concrete. The price shall include the cost of furnishing all equipment, labor, and materials, except dowels and anchors, and all other incidental costs in connection with completing the item of work as shown and as specified. Payment for dowels and anchors will be made as specified for dowels, anchors, and reinforcing steel bars.

##### 1.3.3.2 Measurement

Measurement of drilling holes and grouting dowels and anchors will be made by the linear meter foot of hole actually drilled and grouted to the depth indicated. Measurement will be made from the surface of the concrete after removal or surface preparation and will be made to the nearest 25 mm (inch) inch. No payment will be made for holes improperly drilled or grouted nor for repairs required due to damage caused by the drilling and grouting operations.

##### 1.3.3.3 Unit of Measure

Unit of measure: linear meterfeet.

#### 1.3.4 Dowels and Anchors

Measurement and payment for furnishing and placing dowels, anchors and reinforcing steel bars for anchorage will be made as specified in Section 03201 STEEL BARS AND WELDED WIRE FABRIC FOR CONCRETE REINFORCEMENT FOR CIVIL WORKS.

#### 1.3.5 Concrete

Measurement and payment for cement-based concrete will be made as specified in Section [03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [03307 CONCRETE FOR MINOR STRUCTURES].

##### 1.3.5.1 Payment

Payment for [epoxy] [polymer-modified] [fiber-reinforced] concrete will be made at the respective contract unit prices per cubic meter yard. These payments shall constitute full compensation for performing the work and shall include the cost of all labor, equipment and tools, materials and supplies required to complete the concrete work under this contract; except steel reinforcement, and embedded parts which are specified to be paid for separately. No measurement and payment will be made for concrete, as such, which is placed in structures for which payment is made as a lump sum. No separate payment will be made for bonding coats or agents.

##### 1.3.5.2 Measurement

Measurement of concrete will be made on the basis of the actual volume of concrete placed, within the limits of work, as indicated on the contract

drawings. Measurement of concrete placed against the sides of any excavation without the use of intervening forms will be made only within the pay lines of the structure shown on the contract drawings. Measurement will be made in accordance with paragraph FIELD MEASUREMENT. No deductions will be made for rounded or beveled edges or space occupied by metal work, electrical conduits or other materials, nor for individual voids or embedded items which are either less than 0.15 cubic meters (five cubic feet) five cubic feet in volume or 0.10 square meter (one square foot) one square foot in cross section.

#### 1.3.5.3 Unit of Measure

Unit of measure: cubic meter yard.

#### 1.3.6 Mortar

##### 1.3.6.1 Payment

Payment for [cement-based mortar] [epoxy mortar] [polymer-modified mortar] [Rapid-Hardening Repair Mortar] will be made at the respective contract unit prices per cubic meter foot. These payments shall constitute full compensation for performing the work and shall include the cost of all labor, equipment and tools, materials and supplies required to complete the mortar work under this contract; except steel reinforcement, and embedded parts which are specified to be paid for separately. No measurement and payment will be made for mortar, as such, which is placed in structures for which payment is made as a lump sum. No separate payment will be made for bonding coats or agents.

##### 1.3.6.2 Measurement

Measurement of mortar will be made on the basis of the actual volume of mortar placed, within the limits of work, as indicated on the contract drawings. Measurement will be made in accordance with paragraph FIELD MEASUREMENT. No deductions will be made for rounded or beveled edges or space occupied by metal work, electrical conduits or other materials, nor for voids or embedded items.

##### 1.3.6.3 Unit of Measure

Unit of measure: cubic meter foot.

#### 1.3.7 Grout

##### 1.3.7.1 Payment

Payment for [cement-based] [epoxy] [polymer-modified] grout will be made at the respective contract unit prices per cubic meter foot. These payments shall constitute full compensation for performing the work and shall include the cost of all labor, equipment and tools, materials and supplies required to complete the grout work under this contract. No measurement and payment will be made for grout, as such, which is placed in structures for which payment is made as a lump sum. No separate payment will be made for bonding coats or agents.

##### 1.3.7.2 Measurement

Measurement of grout will be made on the basis of the actual volume of grout placed, within the limits of work, as indicated on the contract

drawings. Measurement will be made in accordance with paragraph FIELD MEASUREMENT. No deductions will be made for rounded or beveled edges or space occupied by metal work, electrical conduits or other materials, nor for individual voids or embedded items.

#### 1.3.7.3 Unit of Measure

Unit of measure: cubic meter foot.

#### 1.3.8 Precast Concrete Units

\*\*\*\*\*  
NOTE: Repeat this bid item and its respective subparagraphs for each bid item of precast concrete units, renumbering the bid items appropriately. Unit price bid items should be inserted in paragraph UNIT PRICE BID ITEMS of Section 01270A MEASUREMENT AND PAYMENT. When uniform thickness and construction precast units are used, area measurement is appropriate. If the units are not uniform, either separate area measurements of volume measurements should be specified.  
\*\*\*\*\*

##### 1.3.8.1 Payment

Payment will be made for costs associated with fabricating, transporting and erecting precast concrete units. The payment will include bearing pads, shims, and all temporary supports required for erection. Payment for precast units will be made at the contract price per square meter foot for:

[<Insert Type of Precast Concrete Unit(s) here>]

##### 1.3.8.2 Measurement

Measurement of precast concrete units will be made on the basis of the actual surface area of precast units placed, and accepted, within the limits of work as indicated on the contract drawings. No deductions will be made for beveled edges or space occupied by metal work, or for embedded items.

##### 1.3.8.3 Unit of Measure

Unit of measure: square meter feet.

#### 1.3.9 Overlays

##### 1.3.9.1 Payment

Payment for [cement-based] [polymer-modified] concrete overlay will be made at the respective contract unit prices per square meter yard. These payments shall constitute full compensation for performing the work and shall include the cost of all labor, equipment and tools, materials and supplies required to complete the overlay work under this contract, including surface preparation. No separate payment will be made for bonding agents or primers, when required.

#### 1.3.9.2 Measurement

Measurement of [cement-based] [polymer-modified] concrete overlay will be made on the basis of the actual area of overlay placed, within the limits of work, as indicated on the contract drawings. No deductions will be made for voids or embedded items which are less than 0.09 square meter (one square foot) one square foot in area.

#### 1.3.9.3 Unit of Measure

Unit of measure: square meter yard.

#### 1.3.10 Crack Injection

##### 1.3.10.1 Payment

\*\*\*\*\*  
NOTE: If there is considerable uncertainty concerning the quantity to be used, consideration should be given to using subdivided payment items, with the first amount being the minimum anticipated and subitems to cover unknown quantities above the minimum.  
\*\*\*\*\*

Payment will be made for costs associated with injecting cracks with [ultra-fine cement grout] [epoxy injection adhesive] [polyurethane injection adhesive], including cleaning the cracks, preparing the surface for crack surface sealer, placing crack surface sealer, installing injection ports, and cleanup upon completion of injection. Payment for will be made at the contract price per liter gallon of material injected.

##### 1.3.10.2 Measurement

Measurement of crack injection will be made on the basis of the actual volume of [ultra-fine cement] [epoxy] [polyurethane] material injected, and accepted, within the limits of work as indicated on the contract drawings. No measurement will be made of grout that is released through ports or otherwise wasted.

##### 1.3.10.3 Unit of Measure

Unit of measure: cubic meterfoot.

#### 1.3.11 Crack or Joint Sealing

Measurement and payment for crack or joint sealing, including sawing or routing cracks or joints, removing existing deteriorated sealants and joint materials, surface preparation, installing sealant and backup materials, and cleanup upon completion of sealing will be made as specified in Section 02760 FIELD MOLDED SEALANTS FOR SEALING JOINTS IN RIGID PAVEMENTS.

#### 1.3.12 Notch and Seal Crack Filling

##### 1.3.12.1 Payment

Payment will be made for costs associated with filling cracks with [dry-pack mortar] [polymer-modified mortar], including preparing the cracks. Payment for will be made at the contract price per cubic meter foot of

material placed.

#### 1.3.12.2 Measurement

Measurement of crack filling will be made on the basis of the actual volume of mortar placed, and accepted, within the limits of work as indicated on the contract drawings.

#### 1.3.12.3 Unit of Measure

Unit of measure: cubic meter foot.

#### 1.3.13 Mechanical Anchoring of Cracks

\*\*\*\*\*  
NOTE: When a substantial amount of work involves mechanical anchoring of cracks, consideration may be given to using separate unit prices for drilling, reinforcing or prestressing steel, and miscellaneous metals for anchorages.  
\*\*\*\*\*

##### 1.3.13.1 Payment

Payment will be for costs associated with preparation and installation of mechanical anchorage systems for crack repair.

##### 1.3.13.2 Unit of Measure

Unit of measure: [each] [lump sum].

#### 1.3.14 Crack Stitching

\*\*\*\*\*  
NOTE: When the area of stitching is small and/or well defined, payment may be lump sum.  
\*\*\*\*\*

##### 1.3.14.1 Payment

Payment will be made for costs associated with stitching cracks, including drilling, grouting, and furnishing and installing stitching staples. Payment for will be made at the contract price per stitching staple installed. Payment for sealing cracks will be made as specified for the specific type of crack treatment.

##### [1.3.14.2 Measurement

Measurement of crack stitching will be made on the basis of the actual number of stitching staples installed and accepted, within the limits of work as indicated on the contract drawings.

##### ]1.3.14.3 Unit of Measure

Unit of measure: [each] [lump sum].

### 1.3.15 Drilling and Plugging Cracks

#### 1.3.15.1 Payment

Payment will be made for costs associated with drilling and plugging cracks with [portland cement grout] [polymer-modified mortar], including drilling the cracks and furnishing and placing the [grout] [mortar]. Payment for will be made at the contract price per cubic meter foot of material placed.

#### 1.3.15.2 Measurement

Measurement of drilling and plugging cracks will be made on the basis of the actual volume of [portland cement grout] [polymer-modified mortar] placed, and accepted, within the limits of work as indicated on the contract drawings.

#### 1.3.15.3 Unit of Measure

Unit of measure: cubic meter foot.

### 1.3.16 Gravity Crack Filling

#### 1.3.16.1 Payment

Payment for gravity filling of cracks will be made at the contract unit price per square meter yard. This payments shall constitute full compensation for performing the work and shall include the cost of all labor, equipment and tools, materials and supplies required to complete the gravity filling work under this contract, including surface preparation.

#### 1.3.16.2 Measurement

Measurement of gravity crack filling will be made on the basis of the actual area of concrete surface treated, within the limits of work, as indicated on the contract drawings.

#### 1.3.16.3 Unit of Measure

Unit of measure: square meter yard.

## 1.4 SYSTEM DESCRIPTION

### 1.4.1 Design Requirements

#### 1.4.1.1 Repair Materials

\*\*\*\*\*

NOTE: If higher earlier strength is needed for repairs, revise this paragraph to provide appropriate criteria. The mixture design should maximize aggregate size and minimize the water-cementitious material ratio to reduce shrinkage. Material properties should be coordinated with EM 1110-2-2002 Evaluation and Repair of Concrete Structures. Where other guide specifications are included in the project specification, ensure that appropriate parameters are inserted in the applicable specification. Consideration should be given to including and

alternative age other than 28 days such as 90 days when pozzolan is used to provide more cost effective use of materials. If flowability of the grout is important, use ASTM C 1017/C 1017M for admixture and specify where it is to be used; otherwise use ASTM C 494.

\*\*\*\*\*

a. Concrete. Design the concrete mixtures in accordance with Section [ 03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [03307 CONCRETE FOR MINOR STRUCTURES].

b. Mortar. Design the mortar mixtures to produce material having an average compressive strength of [\_\_\_\_\_] kPa ([\_\_\_\_\_] psi) [\_\_\_\_\_] psi at 28 days of age, determined in conformance with ASTM C 109/C 109M. Design the mixtures to secure an air content by volume of [5] [\_\_\_\_\_] percent, plus or minus 1-1/2 percent, based on measurements made on concrete at the point of placement in conformance with ASTM C 231. The range of slump shall be 13 to 50 mm (1/2 to 2 inches) 1/2 to 2 inches when tested in accordance with ASTM C 143/C 143M except that maximum slump may be increased to 100 mm (4 inches) 4 inches when the Contractor has included an approved water-reducing, high range, admixture conforming to ASTM C 494/C 494M in the mix design. To minimize drying shrinkage, the maximum water-cementitious materials ratio by weight shall be 0.45 and the maximum cement content shall be kept to a minimum to limit water volume. Make mix design studies and tests in accordance with ASTM C 39/C 39M and ASTM C 192/C 192M, and submit the test results for approval.

c. Grout. Design the grout mixtures to produce material having an average compressive strength of [\_\_\_\_\_] kPa ([\_\_\_\_\_] psi) [\_\_\_\_\_] psi at 28 days of age, determined in conformance with ASTM C 109/C 109M. Design the mixtures to secure an air content by volume of [5] [\_\_\_\_\_] percent, plus or minus 1-1/2 percent, based on measurements made on concrete at the point of placement in conformance with ASTM C 231. The range of slump shall be 13 to 50 mm (1/2 to 2 inches) 1/2 to 2 inches when tested in accordance with ASTM C 143/C 143M except that maximum slump may be increased to 100 mm (4 inches) 4 inches when the Contractor has included an approved water-reducing, high range, admixture conforming to [ASTM C 494/C 494M] [ASTM C 1017/C 1017M] in the mix design. To minimize drying shrinkage, the maximum water-cementitious materials ratio by weight shall be 0.45 and the maximum cement content shall be kept to a minimum to limit water volume. Make mix design studies and tests in accordance with ASTM C 39/C 39M and ASTM C 192/C 192M, and submit the test results for approval.

[Provide the services of a technical specialist experienced in using the polymer for repair materials.]

#### 1.4.2 Repair Material Performance Requirements

Design repair materials to conform to the following requirements at 28 days unless otherwise indicated:

### Physical Properties of Cement-Based Repair materials

Property	Test method	Criteria
Tensile Strength (28 days)	COE CRD-C 164	2.8 MPa (minimum)
Modulus of Elasticity	ASTM C 469	24 GPa (maximum)
Coefficient of Thermal Expansion	COE CRD-C 39	12 millionths/degree C (maximum)
Drying Shrinkage (28 days) (1 year)	ASTM C 157/C 157M (modified)	400 millionths (maximum) 1,000 millionths (maximum)
Restrained Shrinkage Cracking Implied strain at 1 year	AASHTO PP34	No cracks within 14 days, 1,000 millionths (maximum)

### Physical Properties of Cement-Based Repair Materials

Property	Test method	Criteria
Tensile Strength (28 days)	COE CRD-C 164	400 psi (minimum)
Modulus of Elasticity	ASTM C 469	3,500,000 psi (maximum)
Coefficient of Thermal Expansion	COE CRD-C 39	6.7 millionths/degree F (maximum)
Drying Shrinkage (28 days) (1 year)	ASTM C 157/C 157M (modified)	400 millionths (maximum) 1,000 millionths (maximum)
Restrained Shrinkage Cracking Implied strain at 1 year	AASHTO PP34	No cracks within 14 days, 1,000 millionths (maximum)

#### Notes on Table:

a. ASTM C 157/C 157M (modified). The modifications to the test method for this requirement are as follows:

(1) Standard specimen size is 76x76x275 mm (3x3x11-1/4 inch) 3x3x11-1/4 inch for concrete mortar expanded with aggregate and mortar.

(2) Remove sample from mold at 23 plus or minus 1/2 hours and make initial comparator reading immediately. (For rapid hardening materials, remove sample from mold at 3 hours and make initial comparator reading.)

(3) The specimens are then stored under the standard conditions at  $23.0 \pm 2$  degree C ( $73 \pm 3$  degrees F)  $73.4 \pm 3$  degrees F and 50 plus or minus 4% RH.

Subsequent comparator readings are to be taken at ages of 3 days, 7

days, 14 days, 1 month, 2 months; measurements shall continue until 90% of ultimate drying shrinkage is reached. Ultimate shrinkage is to be determined as described in ASTM C 596.

## 1.5 SUBMITTALS

\*\*\*\*\*

NOTE: Review submittal description (SD) definitions in Section 01330 SUBMITTAL PROCEDURES and edit the following list to reflect only the submittals required for the project. Submittals should be kept to the minimum required for adequate quality control.

A "G" following a submittal item indicates that the submittal requires Government approval. Some submittals are already marked with a "G". Only delete an existing "G" if the submittal item is not complex and can be reviewed through the Contractor's Quality Control system. Only add a "G" if the submittal is sufficiently important or complex in context of the project.

For submittals requiring Government approval on Army projects, a code of up to three characters within the submittal tags may be used following the "G" designation to indicate the approving authority. Codes for Army projects using the Resident Management System (RMS) are: "AE" for Architect-Engineer; "DO" for District Office (Engineering Division or other organization in the District Office); "AO" for Area Office; "RO" for Resident Office; and "PO" for Project Office. Codes following the "G" typically are not used for Navy, Air Force, and NASA projects.

Choose the first bracketed item for Navy, Air Force and NASA projects, or choose the second bracketed item for Army projects.

\*\*\*\*\*

Government approval is required for submittals with a "G" designation; submittals not having a "G" designation are [for Contractor Quality Control approval.] [for information only. When used, a designation following the "G" designation identifies the office that will review the submittal for the Government.] The following shall be submitted in accordance with Section 01330 SUBMITTAL PROCEDURES:

### SD-01 Preconstruction Submittals

Work Plan[; G][; G, [\_\_\_\_\_]]

Submit the proposed methods for constructing or repairing each type of concrete section. The plan shall include, but shall not be limited to, repair materials to be used with specific information on products and/or constituents, and requirements for handling, storage, etc., equipment to be used, surface preparation, and requirements for placement, finishing, curing and protection specific to the materials used

Demolition Plan[; G][; G, [\_\_\_\_\_]]

Submit the procedures proposed for the accomplishment of the demolition and removal work. The procedures shall provide for safe conduct of the work, including procedures and methods to provide necessary supports, lateral bracing and shoring when required, careful removal and disposition of materials specified to be salvaged, protection of property which is to remain undisturbed, coordination with other work in progress, and timely disconnection of utility services. The procedures shall include a detailed description of the methods and equipment to be used for each operation, and the sequence of operations in accordance with EM 385-1-1. Where applicable, the plan shall include drilling patterns and means of controlling crack propagation. The work plan shall also include an access plan for personnel; [a Lead Protection Plan]; and method of controlling, collecting, and removing debris.

Blasting Plan[; G][; G, [\_\_\_\_\_]]

Submit a detailed plan for blasting, safety, and traffic controls for approval at least [30][60] days prior to performance of any blasting. Submit a detailed blasting plan for each individual blast for approval at least 24 hours prior to drilling for the blast. The blasting plan shall state location, number of holes, size, depth, spacing, loading of individual holes, type of explosives, time delay sequences, and seismographic monitoring to be performed. The plan shall also address coordination with highway, railroad and river traffic, project personnel and the public as applicable.

Water Control Plan[; G][; G, [\_\_\_\_\_]]

Submit a detailed plan for collection, treatment and disposal of wastewater from drilling, sawing and other concrete removal operations at least [30][60] days prior to performance of any operations that produce wastewater. The plan shall include copies of required permits or other evidence of compliance with applicable Federal, State and local laws and regulations.

Erection Plan for Precast Units

Submit a detailed erection plan at least [30][60] days prior to the date that erection of precast units is to begin. This plan shall be in sufficient detail so that adequacy of equipment, techniques and accessories can be determined and comments offered. The plan shall include design calculations for loading and deflection on the precast units. Acceptance of the Contractor's erection plan shall not relieve the contractor of his responsibility for erecting the precast units into position as required by the contract drawings, and these specifications. The plan shall include the procedure of placing the infill concrete, including the positioning of hoppers and chutes, and all forming techniques to be employed.

Alignment Plan for Precast Units

Submit a detailed plan for aligning precast units to the alignment and profile indicated on the contract drawings.

Identify all safety and quality control aspects for performing this alignment operation. Submit with the erection plan.

Contractor Qualifications  
Worker Qualifications  
Blasting Personnel Qualifications

Submit Information, including name, dollar value, date, and point-of-contact for similar projects which demonstrates the required experience and/or training. Submit resumes, pertinent information, past experience, training, and education of all operators of specialized demolition equipment.

#### SD-03 Product Data

Rapid-Hardening Repair Material  
Polymer Modifier  
Latex Bonding Compound  
Polyurethane Injection Adhesive  
Polyester Resin Grout  
Epoxy Resin Materials  
Bond Breaker  
Fiber Reinforcement  
Neoprene Bearing Pads for Precast Units; [ ], [ ]  
Mechanical Anchors  
High Molecular Weight Methacrylate (HMWM) Sealer

Submit manufacturer's literature from suppliers that demonstrates compliance with applicable specifications for the above materials. For proportioned materials, submit three copies of the proposed mix design prior to placement. The mix design shall indicate the weight of each ingredient of the mixture. No concrete shall be placed prior to approval of the proposed mix design. No deviation from the approved job-mix formula will be permitted without prior approval.

#### Testing Technicians

Submit statements that the concrete testing technicians meet the specified requirements.

#### SD-04 Samples

##### Field Molded Sealants and Primer

Provide four liters one gallon of field-molded sealant and 1 L one quart of primer (when primer is recommended by the sealant manufacturer) for testing.

##### [Epoxy-Coated SteelBars

Sample of coating material and 700 g1.5 pounds of patching material shall be submitted with the delivery of the bars.]

#### SD-05 Design Data

##### Repair material mixture proportioning

Submit mixture proportioning and test results.

SD-06 Test Reports

\*\*\*\*\*

**NOTE: Test reports should be required unless the repairs are considered minor and non-critical, in which case manufacturer's certificates could be accepted.**

\*\*\*\*\*

Compression Seals and Lubricant  
Cement-Based Concrete  
Cement-Based Mortar  
Rapid-Hardening Repair Material.  
Dry-Pack Mortar.  
Polymer-Modified Concrete  
Polymer-Modified Mortar

Submit copies of test results, within [24] [\_\_\_\_\_] hours of physical completion of laboratory testing. Manufacturer's certifications may be submitted rather than laboratory test results for proposed materials. Certificates should certify compliance with the appropriate specification referenced herein. Do not place materials without prior approval from the Contracting Officer.

\*\*\*\*\*

**NOTE: Use the following submittal on projects involving injection grouting of cracks, and other projects where the total surface area of the structure to be repaired exceeds 2 square meters 20 square feet.**

\*\*\*\*\*

Sieve analysis test for aggregate

Epoxy resin binder tests  
Epoxy resin grout tests

Seismographic Monitoring Records

Submit records of seismographic monitoring of blasting within 48 hours of blasting. Provide permanent records of seismological measurement of vibrations in the structure caused by the blast. The record shall contain the following minimum data:

- (1) Date and time of blast.
- (2) Location of seismological instruments, manufacture and model No.
- (3) Velocity measured in millimetersinches per second in vertical, longitudinal, and transverse directions.
- (4) Displacement measured in millimetersinches.
- (5) Acceleration measured in millimetersinches per second squared.

(6) Energy ratio computation.

(7) Peak particle velocity.

#### SD-07 Certificates

Submit manufacturer's certifications in lieu of laboratory test results for proposed materials. Certificates should certify compliance with the appropriate specification referenced herein. Do not place materials without prior approval from the Contracting Officer.

Grout Fluidifier  
Aggregate

\*\*\*\*\*  
**NOTE: Use this submittal on small repair projects,  
not requiring injection grouting of cracks, whose  
total surface area to be repaired does not exceed 2  
square meters 20 square feet.**  
\*\*\*\*\*

Epoxy Resin Binder

Epoxy Grout

[Epoxy-Coated Steel Bars

Written certification for coating material and coated bars shall be submitted with the delivery of the bars.]

#### SD-08 Manufacturer's Instructions

Polymer-Modified Mortar  
Polymer-Modified Concrete  
Polymer Modifier  
Epoxy Concrete  
Epoxy Mortar  
Epoxy Grout  
Epoxy Injection Adhesive  
Rapid-Hardening Repair Material  
Polyurethane Injection Adhesive  
High Molecular Weight Methacrylate (HMWM) Sealer

Submit manufacturer's written mixing, application and curing instructions for each type of material.

Manufacturer's Material Safety Data Sheets

Submit manufacturer's Material Safety Data Sheets for epoxies, polyurethanes, and other potentially hazardous materials.

### 1.6 QUALITY ASSURANCE

#### 1.6.1 Qualifications

Submit Contractor qualifications and worker qualifications for approval in accordance with paragraph SUBMITTALS. The submittals shall, where applicable, identify individuals who will be working on this contract and

their relevant experience. Do not make changes in approved personnel without prior approval of the Contracting Officer.

#### 1.6.1.1 Contractor Qualifications

The Contractor performing the repair work shall have been involved in a minimum of [three] [ ] concrete repair projects similar in size and scope to this project for at least [five] [ ] years.

#### 1.6.1.2 Worker Qualifications

Each worker engaged in the use of specialized removal or application equipment, including [saw operators,] [milling machine operators,] [hydromilling equipment operators,] [epoxy] [polyurethane] injection process, shall have satisfactorily completed an instruction program in the operation of the equipment. [The instruction program for workers engaged in the use of grout injection equipment shall have included theory on the nature and causes of cracking in concrete, methods for permanently repairing damaged structural members, the technical aspects of correct material selection and use, and the operation, maintenance, and troubleshooting of equipment used in the repair work.] Each worker engaged in the operation of specialized equipment for the contract work shall have a minimum of three years of experience in the operation of the equipment.

#### 1.6.1.3 Blasting Personnel Qualifications

Provide a Blasting Engineer to supervise the overall job and a Blasting Foreman for each shift that blasting will occur. The Blasting Engineer shall be experienced in the supervision of controlled blasting operations and shall be responsible for the drilling plan, supervising blasting operations, and maintaining required records. The Blasting Engineer does not need to be at the site for the entire blasting operation. The Blasting Engineer and Blasting Foreman shall have a minimum of 10 years experience in controlled blasting.

#### 1.6.1.4 Quality Control Personnel Qualifications

The individuals who sample and test concrete as required in this specification shall have demonstrated a knowledge and ability to perform the necessary test procedures equivalent to the ACI minimum guidelines for certification of Concrete Field Testing Technicians, Grade I.

#### 1.6.2 Regulatory Requirements

Perform all work in accordance with applicable Federal, State and local safety, health and environmental requirements, and EM 385-1-1. The Contractor shall be responsible for obtaining all permits required by Federal, State and local agencies for the performance of the work.

#### 1.6.3 Pre-Construction Conference

\*\*\*\*\*  
NOTE: Appropriate technical representatives for specialized repair materials should be required to meet with the Government and Contractor representatives to ensure that all parties involved are knowledgeable of the material properties and application requirements.  
\*\*\*\*\*

The Contractor shall arrange a pre-constructino conference for [the repair materials] [ ] to ensure that the Contractor personnel understand all aspects of the repair material, its properties and application procedures. The conference shall include the Contracting Officer or authorized representative, the Contractor, and a competent Technical Representative of the material manufacturer. The Technical Representative shall be fully qualified to perform the work.

#### 1.6.4 Repair Material Mixture Proportioning

Submit, at least 15 days before work commences, a repair material mixture proportioning for each use of [polymer-modified mortar] [polymer-modified concrete] [dry-pack mortar]. Test reports shall accompany the mixture proportions. Identify the proposed source of the materials and state the proportions of each constituent. When determining the mixture design, use samples of materials to be used on the job.

a. Trial batches: Trial batches and testing requirements for various repair materials specified shall be the responsibility of the Contractor. The laboratory performing the tests shall be on site and shall conform to ASTM C 1077. Samples of aggregates shall be obtained in accordance with the requirements of ASTM D 75. Samples of materials other than aggregate shall be representative of those proposed for the project and shall be accompanied by the manufacturer's test reports indicating compliance with applicable specified requirements. Trial mixtures having proportions, consistencies, and air content suitable for the work shall be made based on methodology described in ACI 211.1, which will produce a range of strength encompassing those required for the work. The maximum water-cementitious materials ratios required in Section [03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [03307 CONCRETE FOR MINOR STRUCTURES], paragraph MAXIMUM WATER-CEMENTITIOUS MATERIALS RATIO will be converted to a weight ratio of water to cementitious materials.

b. Supporting criteria: Include in the submittal the following data for each trial batch:

- (1) Proportions by weight
- (2) Unit weights and specific gravities of constituents
- (3) Batch weights
- (4) Compressive strengths in accordance with the following:

Material	Specimen Size	Test
Concrete	150 by 300 mm (6 by 12 inch)	6 by 12 inch cylinders
ASTM C 39/C 39M		
Mortar	75 by 150 mm (2 by 6 inch)	3 by 6 inch cylinders
ASTM C 39/C 39M		
Grout	50 mm (2 inch)	2 inch cube
ASTM C 109/C 109M		

- (5) Curing time
- (6) Working time (polymer-modified materials)

(7) Slump

(8) Air content

#### 1.6.5 Test Reports

##### 1.6.5.1 Epoxy Resin Binder

Include the following:

- a. Viscosity
- b. Consistency
- c. Gel time
- d. Absorption
- e. Shrinkage
- f. Thermal compatibility

##### 1.6.5.2 Epoxy Resin Grout

Include the following:

- a. Epoxy number
- b. Consistency
- c. Compressive single shear strength
- d. Pot life

##### 1.6.5.3 [Cement-Based Concrete] [and] [Cement-Based Mortar]

Include the following:

- a. Initial Slump
- b. Slump over Time
- c. Air Content
- d. Compressive Strength at 7, [14,] 28, 56, [and] [90] days.
- e. Water to cementitious materials ratio
- [f. Tensile Strength]
- [g. Flexural Strength]

##### 1.6.5.4 [Polymer-Modified Concrete] [and] [Polymer-Modified Mortar]

Include the following:

- a. Tensile Strength (28 days)
- b. Modulus of Elasticity

- c. Coefficient of Thermal Expansion
- d. Drying Shrinkage
- e. Restrained Shrinkage
- f. Cracking
- g. Implied strain at 1 year

#### 1.6.6 Field Samples

Prepare a work plan describing the methods of concrete removal and repair, including methods, equipment and materials to be used for each feature. Submit the work plan for approval at least 30 days prior to the start of the work, in accordance with paragraph SUBMITTAL REQUIREMENTS. The work plan shall include a description of field demonstrations. Do not commence work until the work plan and field demonstration representative of the type of work are approved.

##### 1.6.6.1 Concrete Removal

\*\*\*\*\*  
**NOTE: The size of the removal demonstration area  
 should be sufficient to show the proposed methods  
 and the results under typical conditions..**  
 \*\*\*\*\*

Prior to commencement of production concrete removal, perform a test break on an area of [sufficient size] [3 m by 3 m (10 feet by 10 feet)10 feet by 10 feet] [ ] to demonstrate that the proposed removal procedure will result in compliance with the specified requirements. The test break shall verify [proper spacing of holes and procedure for control of crack propagation,] [selection of abrasive materials,] [loading and stemming of charges,] and suitability of the equipment to remove the existing materials.

##### 1.6.6.2 Joint and Crack Sealing

Prepare and seal a test section consisting of approximately [61][ ] meters (20 [ ] feet)[20][ ] feet at a location determined by the Contracting Officer to demonstrate the preparatory and application procedures prior to beginning production sealing. Additional test sections may be required by the Contracting Officer. Remove any material that does not comply with the contract requirements and replace at no additional cost to the Government. Test sections that comply with the contract requirements may remain in place and will be included payment for the work. Use the same procedures and materials as used in the successful test section for production work. [The test section may be incorporated in the final work if accepted by the Contracting Officer.]

##### 1.6.6.3 Precast Unit Assembly Test

Perform a demonstration of all methods and techniques to be used for erecting the precast units.

#### 1.6.6.4 Sample Repair Panels

\*\*\*\*\*  
**NOTE: Edit this paragraph as appropriate. Specify  
location for all field test panels. Add  
requirements for mock-ups if applicable.**  
\*\*\*\*\*

Construct field test panels prior to beginning of work using the repair materials and procedures proposed for use on the job, to demonstrate the results to be attained. The panel shall contain reinforcing steel and embedded items as the production work. The quality and appearance of each panel shall be subject to the approval of the Contracting Officer, and, if not judged satisfactory, additional panels shall be constructed until approval is attained. Formed and finished surfaces in the completed structure shall match the quality and appearance of the approved sample repair panel. For wall refacing, construct a minimum of one sample panel at least 1.25 m (4 feet) 4 feet by 1.5 m (5 feet) 5 feet and the same thickness as the production panel to demonstrate each type of formed and unformed finish required. Each panel shall include a full length and full width joint line.

#### 1.7 DELIVERY, STORAGE, AND HANDLING

##### 1.7.1 Packing, Shipping, Handling, and Unloading

Inspect materials delivered to site for damage, unload and store with a minimum of handling. Deliver resin components and aggregate materials in original sealed containers where applicable.

##### 1.7.2 Epoxy-Resin Materials

Deliver epoxy-resin materials to the site in such manner as to avoid damage or loss. Do not allow epoxy-resin materials to freeze. Storage areas shall be in a windowless and weatherproof, but ventilated, insulated noncombustible building, with provision nearby for conditioning the material to 20 degrees C (70 degrees F) 70 degrees F to 30 degrees C (85 degrees F) 85 degrees F for a period of 48 hours prior to use. Store epoxy resin components and aggregate materials in dry covered areas at temperatures below 30 degrees C (90 degrees F) 90 degrees F. Remove unused mixed materials that have reached end of working or pot life from the job site. Use epoxy-resin materials before the expiration date marked on the packaging.

##### 1.7.3 Polymer Materials

Storage areas shall be in a windowless and weatherproof, but ventilated, insulated noncombustible building. Store polymer materials at temperatures between 4.4 degrees C (40 degrees F) 40 degrees F to 30 degrees C (85 degrees F) 85 degrees F. Do not allow polymer materials to freeze.

##### 1.7.4 Chemical Admixtures

Protect chemical admixtures and store and maintain between 5 degrees C (40 degrees F) 40 degrees F to 30 degrees C (90 degrees F) 90 degrees F. Remove from the site any admixtures subjected to temperatures outside this range, or stored for longer than recommended by the manufacturer. Do not use any admixture that has been in storage for longer than recommended by the manufacturer or that has been subjected to freezing in the work. Remove

such materials from the site.

#### 1.7.5 Waste Management and Disposal

Prepare a water control plan to describe methods and equipment to be used for controlling, collecting and disposing of wastewater from the construction operations. Unless otherwise specified, do not permit concrete and other debris to drop into the [water] [ ]. Collect concrete and/or debris and retain near to its point of removal. Describe the method of debris control and removal in the demolition plan. The method of debris control and removal must be approved by the Contracting Officer. Dispose of debris in accordance with Section 01572 CONSTRUCTION AND DEMOLITION WASTE MANAGEMENT. Remove all concrete which falls into the [water] [ ] at no additional cost to the Government. [Do not permit waste water from surface preparation, cleaning, drilling and cutting operations to directly enter the [water] [ ]. Collect wastewater and treat in accordance with Section 01355A ENVIRONMENTAL PROTECTION.]

### 1.8 PROJECT/SITE CONDITIONS

#### 1.8.1 Environmental Requirements

\*\*\*\*\*  
**NOTE: The maximum placement temperature is a function of the humidity in accordance with EM 1110-2-2000 (Table 8-2) for thin repairs or the mass of the placement for larger repairs. Limitations if ready-mix concrete is specified must also be considered**  
\*\*\*\*\*

Do not place repair materials when weather conditions detrimentally affect the quality of the finished product. Do not place cement-based repair materials when the air temperature is below 5 degrees C (40 degrees F) 40 degrees F in the shade. When air temperature is likely to exceed 35 degrees C (90 degrees F), 90 degrees F, the cement-based repair material shall have a temperature not exceeding 35 degrees C (90 degrees F) 90 degrees F when deposited, and the surface of such placed cement-based repair material shall be kept damp with a water fog until the approved curing medium is applied. Do not place polymer-modified repair materials when the air temperature is below 7 degrees C (45 degrees F) 45 degrees F or above 30 degrees C (85 degrees F) 85 degrees F unless approved by the polymer manufacturer. Do not place sealant in joints or cracks when the temperature is below 10 degrees C (50 degrees F) 50 degrees F. Placement restrictions for other materials shall be in accordance with the manufacturer's published literature. Halt work when weather conditions are potentially detrimental to the quality of repairing or bonding concrete. Apply epoxy resin materials only when the contact surfaces are completely dry and if the ambient and surface temperature ranges are suitable for the specified epoxy material. Follow manufacturer's instructions for weather conditions and temperature ranges.

#### 1.8.2 Existing Conditions

##### 1.8.2.1 Concrete Test Data

\*\*\*\*\*  
**NOTE: Test information and evaluation reports should be included with the contract package if applicable.**

When a substantial amount of information is available, a summary of pertinent information should be included and the remaining information should be made available for review.

\*\*\*\*\*

The existing concrete has been evaluated by means of [core drilling and destructive testing] [petrographic examination] [determination of rebound number] [determination of penetration resistance] [ultrasonic pulse-velocity evaluation]. The results of the evaluation are [given in \_\_\_\_\_] [available for review at \_\_\_\_\_].

#### 1.8.2.2 Concrete Core(s)

Concrete core(s) obtained from the structure, which was (were) not destroyed by testing, is (are) available for viewing at [ ].

#### 1.8.2.3 Embedded Materials

The contract drawings and reference drawings do not constitute a complete description of all metal parts and other materials that may be encountered, but represent the best information available to the Government. Other items, or different locations for items shown, may exist. Exercise care to avoid drilling through functional embedded items intended to remain in service. The Contractor's selection of equipment and methods shall consider the presence of such materials, and the Government will not be responsible in any way for the effect of such items on the Contractor's equipment or progress. Where indicated, remove existing metal items to the limits noted on drawings.

### 1.9 SEQUENCING AND SCHEDULING

Unless otherwise specified, perform the work in such sequence that new work does not damage previously completed work. Do not perform concrete removal and other operations which cause vibrations within 15 [ ] m (50 [ ] feet) 50 [ ] feet of repair materials that have cured less than 24 hours. Provide protective measures as required to protect completed work.

### 1.10 CONSTRUCTION TOLERANCES

#### 1.10.1 Concrete Removal

\*\*\*\*\*

**NOTE: One-inch tolerance is suitable for concrete removal over six inches deep. For thinner repairs, a tighter tolerance would be more appropriate. The methods of removal should also be considered. The relative costs of achieving closer tolerance should be weighed against the anticipated benefits.**

\*\*\*\*\*

Remove concrete to the lines of removal indicated on the contract drawings.

A tolerance of plus or minus [25 mm (1 inch) 1 inch] [13 mm (1/2 inch) 1/2 inch] will be allowed about the lines indicated [except in the thin wall areas where no removal beyond that indicated will be permitted]. Concrete removals that vary by more than the indicated tolerance will be acceptable, unless evidence indicates that deteriorated concrete is still present or the variation in tolerance results in a violation of the requirements for material placement, in which case removal shall continue to the lines

indicated. Additional concrete removal beyond the allowable tolerance will be acceptable when deteriorated concrete is found as determined in accordance with paragraph Determination of Removal Limits, and the limits of such removal are set and approved by the Contracting Officer. Unauthorized concrete removals in excess of 25 mm (1 inch) 1 inch beyond the lines shown will not be acceptable, and repair of such excess area shall be performed by the Contractor to the satisfaction of the Contracting Officer and at no additional cost to the Government.

#### 1.10.2 Repair Materials and Products

Place repair materials to the lines indicated on the drawings. Construction tolerances for concrete and mortar repair materials shall meet the requirements of ACI 117. Construction tolerances for erection of precast concrete units shall meet the requirements of PCI MNL-116.

### 1.11 FIELD MEASUREMENTS

#### 1.11.1 Measurement of Vertical Concrete Removal

Determine the volume of vertical concrete removal from cross sections made prior to and after removal. Make cross sections from surveyed measurements [on a 1.5 meter by 1.5 meter (5 foot by 5 foot) 5 foot by 5 foot grid system] [at a minimum of 20 points], unless otherwise specified by the Contracting Officer. Take initial survey of the existing surface prior to removal, and take final surveys on the same control stations. Calculate volumes by the average-end-area method using the cross sections surveyed, unless otherwise specified. For areas of vertical concrete removal which are not readily accessible for survey, determine the volume of concrete from the neat lines of concrete removal as shown on the contract drawings, with the approval of the Contracting Officer,. Do not include any volume of concrete removal made beyond the allowable tolerances specified in paragraph CONSTRUCTION TOLERANCES.

#### 1.11.2 Measurement of Horizontal Concrete Removal

Determine volumes of horizontal concrete removal from cross sections made prior to and after removal. Make cross sections from surveyed measurements [on a 3 meter by 3 meter (10 foot by 10 foot) 10 foot by 10 foot grid system] [at a minimum of 20 points], unless otherwise specified by the Contracting Officer. Take initial survey of the existing surface prior to removal, and take final cross sections at the same control stations. Calculate volumes by the average-end-area method using the cross sections surveyed, unless otherwise specified. Do not include any volume of concrete removal made beyond the allowable tolerances specified in paragraph CONSTRUCTION TOLERANCES.

#### 1.11.3 Measurements of Concrete

For areas that are readily accessible by a survey crew, determine volumes by cross sections of the areas prior to and after concrete placement. Take the initial and final cross sections at the same control stations used for concrete removal where possible. Calculate volumes by the average-end-area method using the cross sections surveyed, unless otherwise specified. For areas of concrete placement which are not readily accessible for survey, determine the volume of concrete from the neat lines of concrete placement as shown on the contract drawings. Do not include any volume of concrete placed to compensate for removal made beyond the allowable tolerances specified in paragraph CONSTRUCTION TOLERANCES.

## 1.12 RELATED WORK SPECIFIED ELSEWHERE

### 1.12.1 Formwork

Formwork shall conform to the requirements of Section 03101A FORMWORK FOR CONCRETE. Do not use form oil with epoxy or polymer-modified repair materials.

### 1.12.2 Reinforcing Steel

Reinforcing Steel shall conform to the requirements of Section 03201 STEEL BARS AND WELDED WIRE FABRIC FOR CONCRETE REINFORCEMENT FOR CIVIL WORKS.

### 1.12.3 Field-Molded Sealants

Field-molded sealants shall conform to the requirements of Section 03151A EXPANSION, CONTRACTION AND CONSTRUCTION JOINTS IN CONCRETE FOR CIVIL WORKS.

### 1.12.4 Compression Seals

Compression seals shall conform to the requirements of Section 03151A EXPANSION, CONTRACTION AND CONSTRUCTION JOINTS IN CONCRETE FOR CIVIL WORKS.

### 1.12.5 Concrete

Unless otherwise specified, concrete shall conform to the requirements of Section [03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [Section 03307 CONCRETE FOR MINOR STRUCTURES].

### 1.12.6 Shotcrete

Shotcrete shall conform to the requirements of Section 03371 SHOTCRETE.

### 1.12.7 Preplaced-Aggregate Concrete

Preplaced-aggregate concrete shall conform to the requirements of Section 03372 PREPLACED-AGGREGATE CONCRETE.

### 1.12.8 Precast Concrete

Precast concrete shall conform to the requirements of Section 03410 PRECAST[ PRESTRESSED] STRUCTURAL CONCRETE.

### 1.12.9 Disposal of Waste Materials

Dispose waste materials in accordance with the requirements of Section 01572 CONSTRUCTION AND DEMOLITION WASTE MANAGEMENT. Dispose of waste water in accordance with 01355A ENVIRONMENTAL PROTECTION.

## 1.13 SAFETY

Perform all work performed in accordance with EM 385-1-1. To protect personnel from overexposure to toxic materials, conform to the applicable manufacturer's Material Safety Data Sheets (MSDS) or local regulation.

## PART 2 PRODUCTS

### 2.1 MATERIALS

\*\*\*\*\*  
NOTE: Material selection, including type of cement and other cementitious materials, aggregates and admixtures should be in accordance with EM 1110-2-2000 and EM 1110-2-2002.  
\*\*\*\*\*

#### 2.1.1 Cement-Based Materials

\*\*\*\*\*  
NOTE: See the appropriate concrete aggregates design memorandum or thermal study to select the proper requirements for cementitious materials options, pozzolan, and silica fume and insert in the referenced sections as applicable. Determination of the type of cement, including optional physical and chemical requirements, must be based on consideration of environmental exposure, material reactivity and heat of hydration  
\*\*\*\*\*

Cementitious materials shall meet the requirements specified in Section [ 03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [03307 CONCRETE FOR MINOR STRUCTURES].

##### 2.1.1.1 Rapid-Hardening Repair Material

\*\*\*\*\*  
NOTE: ASTM C 928 does not provide requirements for for freeze-thaw durability, for sulphate exposure or alkali reactivity. These materials should be used only when the properties are needed, and then only when a service record for the proposed material, in the same environment, is available or when Government testing is performed. Use the bracketed requirements when necessary for the exposure. See Table 1 in ASTM C 928 for properties of Type R1, R2 and R3.  
\*\*\*\*\*

Prepackaged or site-mixed material: ASTM C 928, Type [R1] [R2] [R3], [with Durability Factor of 50 or more when subjected to freezing and thawing in accordance with ASTM C 666/C 666M, Procedure A] [and] [length change of not more than [ ]% when tested for sulfate expansion in accordance with ASTM C 1012]. Use bonding agent as recommended by the manufacturer.

##### 2.1.1.2 Ultra-fine Cement

Maximum particle size shall be less than 10 microns. The ultra-fine cement shall contain a dispersing agent.

##### 2.1.2 Admixtures

\*\*\*\*\*  
NOTE: High-range water reducers are only appropriate  
\*\*\*\*\*

for high slump (6"150 mm+) mixtures. Accelerating or retarding admixtures should not be used unless specific project requirements justify their use. Accelerators may promote more cracking, and the retarder may make the concrete susceptible to damage, settlement, etc.

\*\*\*\*\*

Admixtures to be used, when required or permitted, shall conform to the appropriate specification listed. Admixtures shall meet the requirements specified in Section [03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [03307 CONCRETE FOR MINOR STRUCTURES].

#### 2.1.2.1 Grout Fluidifier

ASTM C 937.

#### 2.1.3 Aggregate

\*\*\*\*\*

NOTE: Aggregate quality requirements shall be selected based on exposure and environmental conditions, and shall consider quality issues and potential material reactivity for the locality where the work is to be performed.

\*\*\*\*\*

\*\*\*\*\*

NOTE: The maximum practical size of coarse aggregate should be used. The maximum size aggregate should be based on depth of concrete removal, as well as reinforcing and form clearances.

\*\*\*\*\*

Aggregates shall meet the quality and grading requirements of [ASTM C 33 Class Designations 4M or better] [or] [state highway department specification] and the following specific requirements. For material passing No. 200 sieve provide a non-plastic material composed of a minimum of 75 percent limestone dust, talc or silica inert filler. Provide dry aggregate.

- a. For epoxy concrete: ASTM C 33, [maximum size [\_\_\_\_\_] [13 mm (1/2 inch)] [1/2 inch].]
- b. For epoxy mortar: ASTM C 144, [maximum size [\_\_\_\_\_] mm] [\_\_\_\_\_] inch [.] [No. 8 sieve] [No. 40 sieve] [[\_\_\_\_\_] sieve].
- c. For dry-pack mortar: ASTM C 144, maximum size No. 16 sieve
- d. For polymer-modified concrete: ASTM C 33, Size No. 8.
- e. For polymer-modified mortar: ASTM C 144, [maximum size [\_\_\_\_\_] mm] [\_\_\_\_\_] inch [.] [No. 8 sieve] [No. 40 sieve] [[\_\_\_\_\_] sieve].

#### 2.1.4 Water

Water for cleaning, mixing and curing shall be fresh, clean, potable, and free of injurious amounts of oil, acid, salt, or alkali, except that

non-potable water may be used if it meets the requirements of COE CRD-C 400.

#### 2.1.5 Fiber Reinforcement

\*\*\*\*\*  
**NOTE: For nominal maximum aggregate size less than  
40 mm1-1/2 inch, consider using fibers which are  
slightly larger than the maximum aggregate size.**  
\*\*\*\*\*

[Fiber reinforcement shall be fibrillated polyolefin fiber made from virgin polypropylene meeting the requirements for Type III fibers per ASTM C 1116.] [Fiber reinforcement shall be steel deformed type meeting the requirements of ASTM A 820, Type I or II. The aspect ratio (length divided by diameter or equivalent diameter) of the fibers shall be between 45 and 100.] Length of fibers shall be 40 mm (1-1/2 inch)1-1/2 inch.

#### 2.1.6 Polymer Materials

##### 2.1.6.1 Polymer Modifier for Concrete or Mortar

ASTM C 1438, Type II.

##### 2.1.6.2 Polyurethane Injection Adhesive

\*\*\*\*\*  
**NOTE: Polyurethane injection adhesive is intended  
for sealing non-structural cracks. Polyurethane  
injection adhesive is not intended for, and should  
not be used for, structural repairs.**  
\*\*\*\*\*

Two part system composed of polyurethane resin and water. Polyurethane resin shall be 100% hydrophilic resin, capable of forming either a flexible closed-cell foam or cured gel when mixed with water. When mixed with water, the resin shall meet the following requirements.

#### Performance Criteria Properties of the Cured Polyurethane Injection Grout

Property	Test Method	Criteria
Tensile Strength	ASTM D 1623	138 kPa (minimum)
Elongation	ASTM D 1623	400% (minimum)
Bond to Concrete (wet)	ASTM C 273	138 kPa (minimum)

#### Performance Criteria Properties of the Cured Polyurethane Injection Grout

Property	Test Method	Criteria
Tensile Strength	ASTM D 1623	20 psi (minimum)
Elongation	ASTM D 1623	400% (minimum)
Bond to Concrete (wet)	ASTM C 273	20 psi (minimum)

##### 2.1.6.3 Polyester Resin Grout

\*\*\*\*\*

NOTE: Polyester resin grout should not be used for anchors installed in wet holes unless the two-step procedure specified in paragraph ANCHORING AND REINFORCING is used. Single stage grouting can be accomplished with polyester resin grout by using fast setting resin grout cartridges or capsules in the bond zone and slower setting resin grout in the free stressing zone.

\*\*\*\*\*

Polyester resin grout consisting of high strength, unsaturated polyester resin filled with nonreactive, inorganic aggregate and a separated catalyst contained in a cartridge of polyester film or glass capsule. Gel time and cure time shall be appropriate for the installation procedures. The polyester resin grout shall have the following minimum properties:

Compressive Strength - 83 MPa (12000 psi)12000 psi.  
Tensile Strength - 27.6 MPa (4000 psi)4000 psi.  
Shear Strength - 20.7 MPa (3000 psi)3000 psi.

Do not use resin cartridges or capsules with expired shelf life.

#### 2.1.6.4 Latex Bonding Compound

Latex bonding compound agents for bonding fresh to hardened concrete shall conform to ASTM C 1059, Type II.

#### 2.1.6.5 High Molecular Weight Methacrylate (HMWM) Sealer

High molecular weight methacrylate shall be a 2-component, rapid curing, solvent-free, penetrating sealer with components meeting the following requirements:. Sand for covering the HMWM shall meet the quality and grading requirements of aggregate for masonry mortar per ASTM C 144.

a. HMWM Monomer. The monomer shall be a high molecular weight or substituted methacrylate that conforms the following properties:

Property	Physical Properties of HMWH Monomer	
	Test Method	Criteria
Vapor Pressure	ASTM D 323	Less than 133 Pa at 25 degrees C
Flash Point	Pensky-Martens CC	Greater than 93 degrees C
Density		Greater than 1.0 g per cubic cm at 25 degrees C
Viscosity	ASTM D 4016	.012 $\pm$ .004 Pas at 23 degrees C
Index of Refraction		1.470 $\pm$ 0.002
Boiling point @ 133 Pa		70 degrees C
Shrinkage on cure		less than 11 %
Glass Transition Temperature (DSC)	ASTM D 3418	57.2 degrees C
Curing Time (100 g mass)	ASTM D 2471	Greater than 40 minutes at 25 degrees C, with 4% cuemene hydroperoxide
Bond Strength	ASTM C 882	Greater than 10.3 mPa

Physical Properties of HMWH Monomer		
Property	Test Method	Criteria
Vapor Pressure	ASTM D 323	Less than 0.02 psi at 77 degrees F
Flash Point	Pensky-Martens CC	Greater than 200 degrees F
Density		Greater than 8.4 lb. per gal. at 77 degrees F
Viscosity	ASTM D 4016	12 plus or minus 4 cps at 73 degrees F
Index of Refraction		1.470 plus or minus 0.002
Boiling point @ 0.02 psi		158 degrees F
Shrinkage on cure		less than 11 %
Glass Transition	ASTM D 3418	158 degrees F
Temperature (DSC)		
Curing Time	ASTM D 2471	Greater than 40 minutes at 73 degrees F, with 4% cuemene hydroperoxide
(100 g mass)		
Bond Strength	ASTM C 882	Greater than 1,500 psi

b. Initiator/Promoter System

Initiator Cuemene Hydroperoxide	78 percent
Promoter Cobalt Napthenate	6 percent

The initiator/promoter system shall be capable of providing a surface cure time of not less than 40 minutes nor more than 3 hours at the surface temperature of the concrete during application. The initiator/promoter system shall be such that the gel time may be adjusted to compensate for changes in temperature that may occur throughout the treatment application.

2.1.7 Epoxy Resin Materials

\*\*\*\*\*

NOTE: The grades for ASTM C 881/C 881M and classes for both standards are as follows:

1. Grade 1: Low viscosity
2. Grade 2: Medium viscosity
3. Grade 3: Non-sagging consistency
4. Class B: For use between 5 and 15 degrees C 40 and 60 degrees F.
5. Class C: For use above 15 degrees C 60 degrees F.

\*\*\*\*\*

\*\*\*\*\*

NOTE: Epoxy mortars and concrete do not have thermal characteristics compatible with portland cement concrete. This thermal incompatibility should be carefully considered before specifying epoxy mortar or concrete for exterior concrete surface repairs.

\*\*\*\*\*

Epoxy compounds for repairs shall be moisture insensitive.

#### 2.1.7.1 Epoxy Resin Binder for Concrete and Mortar

ASTM C 881/C 881M, Type III, Grade [1] [2], Class [B] [C] without mineral filler. [For [vertical] [and] [overhead] use ASTM C 881/C 881M, Type III, Grade 3, Class [B] [C] with filler.]

#### 2.1.7.2 Non-Pressure Epoxy Grout

ASTM C 881/C 881M Type IV, Grade [2] [3], Class [B] [C] with or without mineral filler. For setting anchors and dowels, use Type III, Grade [2] [3] with mineral filler.

#### 2.1.7.3 Epoxy Injection Adhesive

ASTM C 881/C 881M, Type IV, Grade 1, Class [B] [C] without filler.

#### 2.1.7.4 Crack Surface Sealer for Injection Grouting

ASTM C 881/C 881M, Type IV, Grade 3, Class [B] [C] with mineral filler.

#### 2.1.7.5 Epoxy Bonding Agent

ASTM C 881/C 881M, Type II or V. Type II material for non-loadbearing applications. Type V material for load bearing applications.

#### 2.1.8 Bond Breaker

\*\*\*\*\*  
**NOTE: ASTM D 2822 is an asphalt roofing cement, ASTM D 226 and ASTM D 4869 are asphalt saturated organic felt. ASTM D 2103 is polyethylene sheet. AASHTO M 288 is geotextile. ASTM D 450 is a bituminous coating.**  
\*\*\*\*\*

[ASTM D 2822], [ASTM D 4869], [ASTM D 226, Type I] ,ASTM D 2103, minimum thickness 0.25 mm0.010 inch, [AASHTO M 288, Erosion Control, Class B], [ASTM D 450, Type II].

#### 2.1.9 Field-Molded Sealants and Primer

\*\*\*\*\*  
**NOTE: Verify with ACI 504 that sealants are appropriate for the project requirements.**  
\*\*\*\*\*

Sealants and primers shall meet the requirements specified in Section [03151A EXPANSION, CONTRACTION AND CONSTRUCTION JOINTS IN CONCRETE FOR CIVIL WORKS] [03307 CONCRETE FOR MINOR STRUCTURES].

#### 2.1.10 High-Strength Steel Bars

ASTM A 722/A 722M, Type [I] [or] [II], meeting all supplementary requirements.

#### 2.1.11 Compression Seals and Lubricant

Compression seals shall meet the requirements specified in Section 03151A EXPANSION, CONTRACTION AND CONSTRUCTION JOINTS IN CONCRETE FOR CIVIL WORKS.

#### 2.1.12 Epoxy-Coated Steel Bars

ASTM A 722/A 722M, Type [I] [or] [II], conforming to the coating requirements of ASTM A 775/A 775M, 0.3 mm (8 mils) 8 mils minimum thickness. Coating at the anchorage end may be omitted over the length provided for threading the nut against the bearing plate.

#### 2.1.13 Anchors

ASTM A 615/A 615M, ASTM A 996/A 996M, ASTM A 36/A 36M.

#### 2.1.14 Dowels

ASTM A 615/A 615M, ASTM A 996/A 996M, ASTM A 36/A 36M.

#### 2.1.15 Anchor Head

Anchor head shall consist of steel bearing plate with nut. Anchorage devices shall be capable of developing 95 percent of the guaranteed ultimate strength of prestressing steel. The anchorage devices shall conform to the static strength requirements of Section 3.1.6 (1) and Section 3.1.8 (1) and (2) of PTI A. [Threaded anchorage items for epoxy coated bars shall be designed to fit over the epoxy coating and maintain the capacity of the prestressing steel.]

#### 2.1.16 Mechanical Anchors

[CID A-A-1922] [FS A-A-1923] [CID A-A-55614], galvanized unless otherwise indicated. Provide sleeve anchors of the length and diameter indicated. Minimum concrete embedment shall be as shown. Design values listed shall be as tested according to ASTM E 488.

- a. Minimum [ultimate] [allowable] pullout value shall be [\_\_\_\_\_] kN [\_\_\_\_\_] lb.
- b. Minimum [ultimate] [allowable] shear value shall be [\_\_\_\_\_] kN [\_\_\_\_\_] lb.

### 2.2 ACCESSORIES

#### 2.2.1 Stitching staples

ASTM A 615/A 615M, bar size as shown, with legs bend for embedment as shown on the drawings.

#### 2.2.2 Neoprene Bearing Pads for Precast Units

Molded or extruded polychloroprene, containing reinforcing carbon black, zinc oxide, accelerators, antioxidants, vulcanizing agents, and plasticizer. Pads shall be of the sizes indicated on the contract drawings. The characteristics of the neoprene bearing pads shall meet the following requirements:

Physical Test	Test Method Test Value	Specification
Tensile Strength	8.3 mPa (minimum)	ASTM D 412
Elongation at Break	450 % (minimum)	ASTM D 412

Physical Test	Test Method	Specification
300 % Modulus	Test Value	
Durometer Hardness	6.2 mPa (minimum)	ASTM D 412
Shore Type A	45 to 55 (50 Average)	ASTM D 2240
Water Absorption	5 % by Weight (maximum)	ASTM D 471
Compression Set	30 % (maximum)	ASTM D 395
Tensile Strength	80 % (minimum of tensile after aging 48 hours strength)	ASTM D 572
	Test Method	
Physical Test	Test Value	Specification
Tensile Strength	2,000 psi (minimum)	ASTM D 412
Elongation at Break	450 % (minimum)	ASTM D 412
300 % Modulus	900 psi (minimum)	ASTM D 412
Durometer Hardness		
Shore Type A	45 to 55 (50 Average)	ASTM D 2240
Water Absorption	5 % by Weight (maximum)	ASTM D 471
Compression Set	30 % (maximum)	ASTM D 395
Tensile Strength	80 % (minimum of tensile after aging 48 hours strength)	ASTM D 572

#### 2.2.3 Plastic Shims

Plastic shims for shimming precast concrete units shall conform to ASTM D 4101, polypropylene material, of the widths and lengths as shown on the contract drawings.

#### [2.2.4 Expansion Joint Filler Strips, Premolded

\*\*\*\*\*  
**NOTE: Delete this paragraph if Section 03151A  
EXPANSION, CONTRACTION AND CONSTRUCTION JOINTS IN  
CONCRETE is used.**  
\*\*\*\*\*

Expansion joint filler strips shall meet the requirements specified in Section 03151A EXPANSION, CONTRACTION AND CONSTRUCTION JOINTS IN CONCRETE.

#### ]2.3 MIXTURE PROPORTIONING

##### 2.3.1 Cement-Based Concrete

Mixes for cement-based concrete shall be in accordance with paragraph DESIGN REQUIREMENTS and the applicable provisions of Section [03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [Section 03307 CONCRETE FOR MINOR STRUCTURES]. Approved batch tickets shall be furnished for each load of ready-mixed concrete.

##### 2.3.2 Fiber-Reinforced Concrete

Mixtures for fiber-reinforced concrete shall be in accordance with ASTM C 1116, paragraph DESIGN REQUIREMENTS and the applicable provisions of Section [03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [Section 03307 CONCRETE FOR MINOR STRUCTURES].

### 2.3.3 Dry-Pack Mortar

\*\*\*\*\*  
**NOTE: For severe conditions, the water-cementitious materials ratio may need to be 0.40 or less. Dry-pack mortar may be polymer-modified when appropriate.**  
\*\*\*\*\*

Prepare mortar consisting of one part portland cement, three parts fine sand which passes a No. 16 sieve, [polymer proportioned as recommended by the manufacturer, ]and only enough water so the mortar will stick together in a ball when molded by hand. The water-cementitious materials ratio shall not be greater than [0.45][ ] by weight. Let mortar set 1/2 hour prior to placing.

### 2.3.4 Polymer-Modified Mortar

Polymer-modified mortar shall consist of portland cement, fine aggregate, water, and polymer. Proportion mortar in accordance with polymer manufacturer's recommendations. Cured mortar shall comply with the requirements of paragraph REPAIR MATERIAL PERFORMANCE REQUIREMENTS.

### 2.3.5 Polymer-Modified Concrete

Polymer-modified concrete shall consist of portland cement, fine and coarse aggregate, water, and polymer. Proportion concrete in accordance with polymer manufacturer's recommendations. Cured concrete shall comply with the requirements of paragraph REPAIR MATERIAL PERFORMANCE REQUIREMENTS. Polymer-modified concrete for overlays shall conform to ACI 548.4.

### 2.3.6 Precast Concrete Units

\*\*\*\*\*  
**NOTE: Consider use of harder aggregates and pozzolan in the design of precast concrete to assure that the units are durable and abrasion resistant.**  
\*\*\*\*\*

Cement for precast concrete may be Type I portland cement or Type III high early strength portland cement in accordance with ASTM C 150. If using Type III cement, make all necessary adjustments pursuant to ACI requirements to assure the dimensional stability of the units.

### 2.3.7 Cement-Based Grout

Cement-based grout shall consist of equal parts of Type [I or II], [III] portland cement [and] sand by dry weight, [and water-reducing admixture, ]thoroughly mixed with water to yield a thick, creamy mixture. The water-cementitious materials ratio shall not be greater than 0.45 by weight. The sand shall meet the requirements of the fine aggregate specified herein, except 100 percent shall pass a 2.36 mm (No. 8) No. 8 sieve.

### 2.3.8 Ultra-fine Cement Grout

Proportion the ultra-fine cement grout to eliminate shrinkage. The water/cement ratio shall be by volume and shall not exceed manufacturer recommendations for the required strength. Proportion the grout to limit

bleed water. The final bleeding in accordance with ASTM C 940 shall be less than 2%. The expansion in accordance with ASTM C 940 shall be 0 to 5%. Mix the ultra-fine cement using a high shear colloidal mixer.

## PART 3 EXECUTION

### 3.1 EQUIPMENT

Assemble at the site of the work sufficient equipment that is dependable, appropriate and adequate to accomplish the work specified. Deliver the equipment a sufficient time before the start of repairs to permit thorough inspection, calibration of weighing and measuring devices, adjustment of parts, and the making of any repairs that may be required. Machines, tools, and equipment used in the performance of the work shall be approved before the work is started and shall be maintained in satisfactory condition at all times. Maintain the equipment in good working condition. Provide dust suppression on equipment as needed to comply with Section 01355A ENVIRONMENTAL PROTECTION.

#### 3.1.1 Cement-Based Concrete Mixing Equipment

Mixing equipment shall conform to the requirements of Section [03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [Section 03307 CONCRETE FOR MINOR STRUCTURES].

#### 3.1.2 Polymer-Modified Concrete Mixing Equipment

Use proportioning and mixing equipment with capacity and continuous rate so that final finishing can be completed prior to the formation of a plastic film on the polymer-modified concrete surface. The equipment shall consistently produce a uniformly blended mixture within the specified air content and slump limits. The mixer shall also:

- a. Be equipped with a recording meter with a ticket printout device to record an indication of the cement quantity being introduced into the mix. The metering device shall be accurate within a tolerance of -1 to +3 percent.
- b. Be equipped with a polymer metering device to indicate volume dispensed. The metering device shall be accurate within a tolerance of -1 to +2 percent. In addition the polymer tank shall have a stand pipe marked in litersgallons.
- c. Be equipped with a water flow indicator, and have a water flow control that is readily adjustable to provide for minor variations in aggregate moisture content. The flow indicator shall be accurate within a tolerance of plus or minus 1 percent in the range of expected use.
- d. Be equipped with a control to regulate the quantity of each of the polymer-modified concrete components to permit production of a mix having the specified composition. To ensure that the mixer can accurately proportion and blend all components of the polymer-modified concrete on a continuous or intermittent basis, the mixer shall be calibrated prior to the start of the overlay placement.

#### 3.1.3 Epoxy Mixing Equipment

Use a container recommended by the epoxy manufacturer as the mixing vessel.

Use a power drive (air or spark-proof) propeller type blade for mixing except that hand mixing may be used for small batches. Use equipment specified by epoxy manufacturer for field mixing of aggregates and epoxy resin.

#### 3.1.1.4 Grout Mixing Equipment

Use a high-speed, high-shear, colloidal type grout mixer capable of continuous mechanical mixing that will produce uniform and thoroughly mixed grout which is free of lumps and undispersed cement. The mixer shall be equipped with a suitable water [and admixture] measuring device[s] calibrated to read in cubic centimeters/cubic feet and tenths and so designed that after each delivery the device can be conveniently set back to zero.

#### 3.1.1.5 Joint and Crack [Cleaning] and [Sealing] Equipment

Joint and crack [cleaning] and [sealing] equipment shall meet the requirements specified in [Section 02760 FIELD MOLDED SEALANTS FOR SEALING JOINTS IN RIGID PAVEMENTS] [Section 03151A EXPANSION, CONTRACTION AND CONSTRUCTION JOINTS IN CONCRETE FOR CIVIL WORKS].

#### 3.1.1.6 Drilling Equipment

Use percussion or rotary drilling equipment of a type suitable for the depth, diameter and material to be drilled. Use only rotary drilling equipment where vibration from percussion drilling could damage the concrete to remain or adjacent structures. Use equipment capable of maintaining the required alignment.

#### 3.1.1.7 Vehicle-Mounted Breakers

\*\*\*\*\*  
**NOTE: The energy of breakers should be limited to prevent damage to concrete and other structures which are to remain. Verify that the stated energy limits are appropriate for the project requirements**  
\*\*\*\*\*

Use boom or vehicle mounted impact breakers, spring-action hammers or drop balls with blow energy not exceeding approximately [205] [ ] newton-meters ([150] [ ] ft-lbs) [150] [ ] ft-lbs.

#### 3.1.1.8 Hand-Held Breaker

Use 14 kg (30 pound) 30 pound breaker for removal of concrete above reinforcing steel. Use 7 kg (15 pound) 15 pound chipping hammer to remove concrete around reinforcing steel.

#### 3.1.1.9 Hydromilling Equipment

Use hydromilling equipment consisting of filtering and pumping units operating in conjunction with a remote-controlled robotic device or beam. The equipment shall operate at a noise level of less than 90 decibels at a distance of 15 meters (50 feet) 50 feet from the equipment. The equipment shall produce a highly roughened, bondable surface.

### 3.1.10 Rotary Milling Equipment

#### 3.1.10.1 Horizontal Surfaces

Use milling equipment for horizontal surfaces consisting of a self-propelled unit with rotary cutter head with tungsten-carbide bits.

#### 3.1.10.2 Vertical Surfaces

Use rotary equipment consisting of a boom-mounted rotary cutter.

### 3.2 PROTECTION

#### 3.2.1 Protection of Existing Features

Before beginning any concrete removal work, carefully survey the structure and examine the drawings and specifications to determine the extent of the work. Take all necessary precautions to insure against damage to existing concrete or other structures to remain in place, and repair or replace any damage to such items as approved by the Contracting Officer at no additional cost to the Government. Carefully coordinate the work of this section with all other work, and construct and maintain shoring, bracing and supports, as required. Insure that structural elements are not overloaded, and increase structural supports or add new supports as may be required as a result of any removal work performed under any part of this contract.

#### 3.2.2 Protection of Personnel

Insure that adequate measures are in place to protect workers, facility operation personnel and the public from injury due to the operations being performed. Provide protective measures in accordance with EM 385-1-1.

### 3.3 REMOVAL OF EXISTING CONCRETE

\*\*\*\*\*  
NOTE: Depending on the method of concrete removal  
and the condition of the existing concrete,  
microfracturing may occur beyond the removal limits.  
Additional concrete removal by other methods such  
as hand-held breakers, bushhammering, or  
high-pressure water may be required to remove some  
of the microfractured concrete.  
\*\*\*\*\*

#### 3.3.1 General

Prepare a demolition plan describing the methods and equipment to be used to remove existing concrete. Remove the existing concrete in the area to be repaired to the minimum depth indicated and to such additional depth where necessary to expose a surface of sound, unweathered concrete that is uncontaminated by oils, greases, or deicing salts or solutions. The actual depth of removal shall be subject to approval by the Contracting Officer. Remove concrete with care to avoid damage to adjacent structures and concrete that is not to be repaired under this contract and embedded metal that is not to be removed. Repair any such damage at no additional cost to the Government. Mark on the surface the limits of concrete removal for approval by the Contracting Officer prior to any removal. Do not remove metal and other embedded items exposed during the concrete removal

operations without authorization of the Contracting Officer. Unless otherwise indicated, continue removal using appropriate equipment to remove unsound concrete and to eliminate any offsets in the area to be repaired which would cause an abrupt change in thickness of the repair and to remove protrusions between holes. Variations in the final prepared surface shall not exceed the nominal maximum size aggregate of the repair material, except for relatively thin repairs in which case the variation shall not exceed 25 percent of the repair thickness. Any removal beyond the limits shown on the drawings shall be approved by the Contracting Officer prior to performing the additional removal. All equipment and removal methods shall comply with applicable sections of EM 385-1-1.

#### 3.3.1.1 Determination of Removal Limits

Approximate locations and areas of repairs are indicated on the drawings. Determine actual locations and limits of deteriorated concrete by visual inspection and by tapping with a hammer or steel rod and listening for dull or hollow sounds. On horizontal surfaces, a chain drag may be used in lieu of the hammer or rod. Perform sounding in the presence of the Contracting Officer or authorized representative. Mark areas where sounding does not produce a solid tone with highly visible paint. After the investigation process is completed, the Contracting Officer will inspect the areas marked by the Contractor to confirm and/or adjust the limits of removal. Do not remove concrete from these areas until the Contracting Officer or authorized representative approves the areas. Upon completion of removal, sound the surface and remove additional concrete until testing produces a solid tone. Additional removal of concrete within these areas will be measured for payment as specified.

#### 3.3.1.2 Perimeter Saw Cut

\*\*\*\*\*  
**NOTE: In general, the minimum depth of saw cut  
should be the nominal maximum size coarse aggregate.**  
\*\*\*\*\*

Make a perpendicular saw cut at least [25] [50] [ ] mm ([1] [2] [ ] inches) [1] [2] [ ] inches deep, but no deeper than the reinforcing steel, a minimum of [25] [ ] mm ([1] [ ] inch) [1] [ ] inch outside of the area to be repaired to delineate the perimeter of the repair area and avoid feather edges. The perimeter saw cut shall have no angles less than 90 degrees. Exercised care not to cut through existing steel reinforcement or embedded metal that is not to be removed. Omit saw-cutting where prohibited by existing metal or other items which are not to be removed. Perform sawing in accordance with CSDA-W-104, except as specified herein. Edges of repaired areas which are not mechanically anchored shall be dove-tailed. Where concrete is being removed from a vertical face by controlled blasting, mechanical presplitting, or expansive agents, sawcut the bottom limit of the removal area to the full depth of the removal.

#### 3.3.1.3 Preliminary Surface Cleaning

\*\*\*\*\*  
**NOTE: Where the depth of removal exceeds the  
anticipated depth of contamination, removal of such  
contamination prior to removal of the concrete is  
not normally necessary. Where depth of removal is  
shallow or where the removal methods may result in  
spreading of contamination or contamination of**

exposed concrete surfaces, the contaminants must be  
remove prior to removal of the concrete.

\*\*\*\*\*

Clean concrete surfaces that are contaminated with oil, grease or dirt prior to beginning concrete removal in the area. Use detergents, proprietary cleaners formulated for removal of contaminants from concrete, or steam cleaning in accordance with the manufacturer's written instructions. Do not use solvents for removal of oil or grease. All traces of cleaning agents and contaminants shall be removed from the surface.

#### 3.3.1.4 Reinforcing Steel

\*\*\*\*\*

NOTE: The deterioration point at which reinforcing steel must be replaced should be made by a structural engineer and should consider the function of the reinforcing steel (i.e. temperature vs. tension). If the referenced specification is not included in the project specification, applicable portions should be inserted here.

\*\*\*\*\*

Following the concrete removal operation, inspect the condition of all exposed reinforcing bars designated to remain in place. Remove and replace bars which are deteriorated at any point where the original cross section of any bar has been reduced by more than [25] [ ] percent or the original cross section of adjacent bars has been reduced by more than [20] [ ] percent. Splice new replacement steel in accordance with the provisions of ACI 318/318R. Secure to adjacent bars all reinforcing steel that is free to vibrate or otherwise move excessively. Concrete reinforcement shall conform to Section 03201 STEEL BARS AND WELDED WIRE FABRIC FOR CONCRETE REINFORCEMENT FOR CIVIL WORKS. Where the bond between the concrete and any reinforcing steel has been destroyed, or where the concrete deterioration is caused by corrosion of the reinforcing steel, remove the adjacent concrete to a depth that will permit cleaning of the steel and bonding of the concrete. Provide a minimum clearance below the steel of 25 mm (1 inch) 1 inch or 6 mm (1/4 inch) 1/4 inch plus the maximum size of aggregate in the repair material, whichever is greater. clearance around the steel, except where other reinforcing steel makes this impracticable. Use chipping hammers weighing less than 7 kg (15 pounds) 15 pounds, abrasive water cutting, or high-pressure water for removal of concrete around reinforcing steel. Remove all corrosion from reinforcing steel by sandblasting.

#### 3.3.1.5 Shallow Repairs

For shallow repairs that do not require reinforcing steel, remove the existing concrete to a minimum depth of [50] [ ] mm ([2] [ ] inches) [2] [ ] inches below the finished surface and to such additional depth where necessary to expose a surface of sound, unweathered concrete that is uncontaminated by oils, greases, or deicing salts or solutions. Remove all loose and deteriorated concrete from the spalled areas indicated.

#### 3.3.2 Cutting

##### 3.3.2.1 General

Use cutting to remove segments of concrete intact or to define the limits

of removal by other methods.

#### [3.3.2.2 Abrasive-Water-Jet Cutting

\*\*\*\*\*  
**NOTE: EM 1110-2-2002 does not consider  
abrasive-water-jet cutting to be cost effective when  
compared to diamond blade cutting. This method  
should only be used when unique project conditions  
require its use.**  
\*\*\*\*\*

Perform abrasive-water-jet cutting by injecting abrasive material into a high pressure stream of water. The water pressure shall be 200 to 340 MPA (30,000 psi to 50,000 psi)30,000 psi to 50,000 psi. Adjust the water pressure and amount of abrasive added as required to provide optimum cutting action.

#### ]3.3.2.3 Diamond-Blade Cutting

Use diamond-blade cutting for cutting through relatively thin sections and defining limits of removal to be performed by other methods. Perform diamond blade cutting using equipment of the correct type and power and with appropriate blade composition for the material being cut. Use a track system for maintaining the alignment of the saw for vertical cuts or where multiple cuts are required for deep cuts. Provide supports as required to support the concrete to be removed. [Core drill corners of removal area or use corner saw to avoid overcutting.]

#### 3.3.2.4 Diamond-Wire Cutting

Perform diamond wire cutting using equipment of the correct type and power using continuous loop cable with appropriate diamond beads for the material being cut. Core drill holes at corners of removal section to the accuracy required for the wire cutting. Provide supports as required to support the concrete to be removed.

#### 3.3.2.5 Stitch Drilling

\*\*\*\*\*  
**NOTE: Rotary-percussion drilling is more efficient  
for unreinforced concrete. Core drilling may be  
required for reinforced concrete or where the  
vibration of rotary-percussion drilling may cause  
damage to adjacent structures or equipment.**  
\*\*\*\*\*

Drill overlapping holes of appropriate size along the removal line. Unless otherwise indicated, drill holes using rotary-percussion or core drilling equipment. Utilize guides as required to maintain alignment. Provide supports as required to support the concrete to be removed.

#### 3.3.3 Impacting

Removal of concrete may be accomplished by use of impact-type power tools. Take adequate precautions to prevent impact equipment from vibrating on reinforcing steel. All demolition equipment shall be subject to approval of the Contracting Officer.

#### 3.3.3.1 Vehicle-Mounted Breaker

\*\*\*\*\*  
NOTE: Vehicle-mounted breakers may result in microfracture damage of the concrete which remains. Additional concrete removal by other methods such as hand-held breakers or high-pressure water may be required to remove some of the microfractured concrete.  
\*\*\*\*\*

Use boom or vehicle mounted impact breakers or spring-action hammers or drop balls to remove concrete in relatively thin sections or to break up concrete which has been separated from the structure by other methods.

#### 3.3.3.2 Hand-Held Breaker

Use hand-held breakers or chipping hammers for removal of concrete in small or restricted areas, where the energy to be applied must be limited to prevent damage to adjacent concrete, structures or facilities which are not to be removed, and to complete removal where other methods have been used.

#### 3.3.4 Milling

##### 3.3.4.1 Hydromilling

Remove concrete from vertical or horizontal surfaces by means of high pressure water applied through nozzles attached to a remotely-controlled device or beam. Adjust water pressure and rate of movement as required to remove concrete to the required depth and avoid excessive removal. Supplement hydromilling with abrasive water-jet, high-pressure wand, or other methods in areas not accessible to the hydromilling equipment. Submit proposed hydromilling equipment and procedures to the Contracting Officer for review and approval before beginning work. Remove slurry from the prepared surface before it hardens.

##### 3.3.4.2 Rotary Head Milling

Use rotary head milling for uniform depth removal on horizontal or vertical surfaces. Use boom-mounted milling head for vertical surfaces. When operating rotary head equipment from top of wall for removal from vertical surfaces, provide communication between observer and operator to aid in control of removal.

#### 3.3.5 Presplitting

\*\*\*\*\*  
NOTE: Presplitting only separates the concrete. Other measures must be employed to remove reinforcing steel and other embedded items. Hole spacing and diameter are critical to proper development of the presplit plane. A test break should be performed to verify the proposed procedure.  
\*\*\*\*\*

##### 3.3.5.1 Expansive Agents

Use an expansive compound, manufactured for concrete and rock demolition. Drill holes, including relief holes, of the size and at the spacing

recommended by the manufacturer. Mix and place the material in accordance with the manufacturer's recommendations. Insert closed-end plastic tubes into each hole prior to placing the expansive material to prevent dilution of the material and to prevent slurry from entering existing cracks.

#### 3.3.5.2 Mechanical Presplitting

Drill holes of size and spacing as recommended by the manufacturer for the conditions present. Insert and operate mechanical splitters in accordance with the manufacturer's instructions.

#### 3.3.6 Controlled Blasting

\*\*\*\*\*  
NOTE: Blasting may result in microfracturing of the concrete which remains. Additional concrete removal by other methods such as hand-held breakers or high-pressure water may be required to remove some of the microfractured concrete.  
\*\*\*\*\*

##### 3.3.6.1 General

Controlled blasting shall consist of a systematic process of sawcutting, drilling, loading and blasting to break the concrete on a controlled plane. Submit a detailed blasting plan in accordance with paragraph SUBMITTALS. All use of explosives shall be subject to prior approval of the Contracting Officer. Do not deviate from the approved plan without authorization of the Contracting Officer. After blasting, submit an "as-built" blasting plan showing any approved changes. Approval of the Contractor's blasting plan shall not relieve him of responsibility for any damages or injuries resulting from blasting.

##### 3.3.6.2 Safety

Handle, store, and use explosives in accordance with applicable provisions of the EM 385-1-1 and state and local regulations. All blasting work shall be at the risk and responsibility of the Contractor. Conduct all blasting operations with due regard to the safety of persons in the vicinity and traffic passing the site. Make provisions for warning personnel and traffic in advance of blasting.

##### 3.3.6.3 Procedure

\*\*\*\*\*  
NOTE: The suggested peak particle velocity applies to concrete adjacent to structures which are to remain. The permissible peak particle velocity should be determined based on the existing materials and conditions to avoid damage to structures and equipment to remain.  
\*\*\*\*\*

Determining the diameter, depth and spacing of holes, and loading, delays and stemmings for explosives. Drill holes with equipment competent to maintain the alignment and plane of the drill hole pattern throughout the full depth of the holes. Limit extent of blasting area and use delays as required, to prevent damage to the structure. Peak particle velocity shall not exceed [50] [ ] mm ([2] [ ] inches)[2] [ ] inches per second.

Airblast pressure shall not exceed 0.7 kPa (0.1 psi)0.1 psi. Should vibration exceed the specified limits, modify the blasting plan as required, subject to approval by the Contracting Officer. Use detonating cord as explosives. Use blasting mats as necessary to control flyrock and contain debris.

#### 3.3.6.4 Preblasting and Postblasting Inspections

Perform a preblast and postblast inspection of all structures within [180] [ ] meters ([600] [ ] feet)[600] [ ] feet of the blasting operations. The inspection shall include preblast and postblast notes, diagrams, and/or tape recorded descriptions of defects in interior walls, ceilings, floors and foundations; photographs and/or videotape recordings of all prominent interior and exterior cracks; notes on walkways, retaining walls, accessible roofs, chimneys, and similar features. Complete the preblast inspection prior to the beginning of any blasting. Complete the postblast inspection immediately upon cessation of all blasting. Notify the Contracting Officer in writing a minimum of [7] [ ] days in advance of the inspections.

#### 3.3.6.5 Vibration Monitoring

Perform vibration monitoring by placing recording devices on adjacent structures as indicated on the drawings. Modifications to the monitoring procedure may be directed by the Contracting Officer. In order to control blasting vibrations within specified limits, provide the services of an approved commercial vibration-testing laboratory. Furnish seismographic monitoring records and written interpretations of such to the Contracting Officer within 48 hours, except that in the event that any record or records indicate the potential for damage or imminent danger, make an immediate verbal report and remedial recommendation. The vibration monitoring record shall include the following minimum data:

- a. Date and time of blast.
- b. Location of seismological instruments, manufacture and Model No.
- c. Velocity measured in mm/inches per second in vertical, longitudinal and transverse directions.
- d. Displacement measured in mm/inches.
- e. Acceleration measured in mm/inches per second squared.
- f. Energy ratio computation.
- g. Peak particle velocity.

#### 3.4 ANCHORING AND REINFORCING

\*\*\*\*\*  
**NOTE: For core drilled holes, use epoxy grout only.**  
\*\*\*\*\*

##### 3.4.1 Drilling and Grouting Dowels and Anchors

Drill holes for dowels and anchors using drilling equipment suitable for the intended purpose, as approved by the Contracting Officer. Diameter of holes shall be [as shown on the drawings] [as recommended by the adhesive manufacturer]. Clean holes by flushing with water and compressed air prior to placing grout. Anchor [dowels] [anchors] with [epoxy grout], [polyester resin grout], [polyester resin grout cartridges] [or] [cement-based grout].

Place grout and install [dowels] [and] [anchors] as recommended by the grout manufacturer. Remove excess grout after the [dowel] [or] [anchor] has been set in place. [When installing resin cartridges or capsules in

submerged conditions, place properly proportioned resin material in bottom of hole using a mixing tube prior to inserting the cartridge or capsule.]

#### 3.4.2 Drilling and Installing Mechanical Anchors

Drill holes for anchors using drilling equipment suitable for the intended purpose, as approved by the Contracting Officer. Diameter of holes shall be [as shown on the drawings] [as recommended by the anchor manufacturer]. Clean holes by flushing with water and compressed air prior to placing grout. Install anchors and set anchors in place in accordance with the manufacturer's recommendations.

#### 3.4.3 Installing Reinforcement

\*\*\*\*\*  
**NOTE: If the referenced specification is not included in the project specification, applicable portions should be inserted here.**  
\*\*\*\*\*

Place reinforcement steel and accessories as specified and as shown on the contract drawings. Placement details of steel and accessories not specified or shown on the drawings shall be in accordance with ACI SP-66 and ACI 318/318R or as directed by the Contracting Officer. Fabricate and place reinforcing steel in accordance with 03201 STEEL BARS AND WELDED WIRE FABRIC FOR CONCRETE REINFORCEMENT FOR CIVIL WORKS.

#### 3.5 MIXING MATERIALS

Make batches small enough to ensure placement before binder sets. Mix materials in accordance with manufacturer's recommendations.

#### 3.6 SURFACE PREPARATION

\*\*\*\*\*  
**NOTE: For additional guidance on surface preparation, see International Concrete Repair Institute Guidelines No. 03732 "Selecting and Specifying Concrete Surface Preparation for Sealers, Coatings, and Polymer Overlays".**  
\*\*\*\*\*

##### 3.6.1 General

After removal of concrete to the specified limits, clean the surface to which the repair material is to be applied to remove dust, debris and laitance. Perform final cleaning immediately prior to placement of the repair material. Unless otherwise specified, keep the existing concrete wet for a minimum of 12 hours and dried for 12 hours immediately prior to placing new concrete. The surface shall be dry with no standing water on the surface upon which concrete is placed.

##### 3.6.2 Cleaning

\*\*\*\*\*  
**NOTE: If the referenced specification is not included in the project specification, applicable portions should be inserted here.**  
\*\*\*\*\*

Perform all cleaning operations to the satisfaction of the Contracting Officer. Protect adjacent structures and embedded items. Use potable water for all cleaning operations. Perform a preliminary washing as soon as the chipping and trimming are completed to remove loose materials and dust particles. Clean surfaces to which new concrete is to be bonded in accordance with Section 03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS, paragraph CONSTRUCTION JOINT TREATMENT. Final cleaning shall remove all laitance, carbonation, scum, dirt, oil, grease, and loose or disintegrated concrete. Perform additional surface chipping to remove coarse aggregate that is undercut by cleaning process. Perform such additional chipping as determined necessary by the Contracting Officer at no additional cost to the Government. Wire brush or sandblast metal surfaces against which concrete is to be placed to remove rust and other contaminants which would prevent proper bond with the concrete. Perform final cleaning immediately prior to concrete placement. Protect all work from contamination during all phases of cleanup and preparation prior to repair.

### 3.6.3 Waste Water Disposal

Dispose of waste water employed in cutting, washing, and rinsing of concrete surfaces in a manner such that the waste water does not stain, discolor, or affect exposed surfaces of the structures, or damage the environment of the project area. The method of disposal shall meet all requirements of Section 01355A ENVIRONMENTAL PROTECTION.

## 3.7 SURFACE REPAIRS

\*\*\*\*\*  
**NOTE: Dry-pack mortar is also typically used for small spalled areas with high depth to surface area ration. Polymer-modified mortar, polymer-modified concrete and cement-based concrete are typically used for larger repair areas.**  
\*\*\*\*\*

Place dowels and anchors as shown on the contract drawings. [Construct formwork in accordance with Section 03101A FORMWORK FOR CONCRETE.]

### 3.7.1 Epoxy Mortar

Prime surfaces with epoxy resin binder. Scrub prime coat into surface with a stiff bristle brush. Make coating approximately 0.5 mm (20 mils) thick. Place epoxy mortar while primer is still tacky. Apply at a thickness recommended by the manufacturer. Work mortar into place and consolidate thoroughly so that contact surfaces are wetted by the mortar. Finish surface of mortar to the required texture. Do not feather edge epoxy mortar onto adjacent surfaces.

### 3.7.2 Epoxy Concrete

\*\*\*\*\*  
**NOTE: Do not place epoxy concrete in layers greater than 25 mm one inch in thickness. This is to avoid heat buildup and subsequent thermal contraction.**  
\*\*\*\*\*

Prime dry cavity surfaces with epoxy resin using a stiff bristle brush.

Make coating approximately 0.5 mm (20 mils) 20 mils thick. Place epoxy concrete while primer is still tacky and in layers not exceeding 25 mm (one inch)one inch thick. Use vibratory floats, plates, or hand tampers to consolidate the concrete. Level each layer and screed the final surface to match the adjoining surfaces. Remove excess epoxy concrete on adjacent surfaces before the concrete hardens. Do not feather epoxy concrete out onto adjacent surfaces.

### 3.7.3 Polymer-Modified Mortar or Concrete

#### 3.7.3.1 Horizontal Surfaces

Prime surfaces prepared to receive repair material using the manufacturer's recommended bonding agent using a stiff bristle brush and in accordance with the manufacturer's instructions. Place polymer-modified concrete before the bonding agent dries and in layers not exceeding 25 mm (one inch) one inch thick for mortar and 50 mm (two inches)two inches thick for concrete. Moist cure polymer-modified material for a minimum of 24 hours but not more than 48 hours, and then air dry.

#### 3.7.3.2 Vertical Surfaces

For vertical surfaces that are troweled follow the procedure for horizontal surfaces. For formed vertical surfaces follow the procedure for cement-based concrete for vertical surfaces. Moist cure polymer-modified material for a minimum of 24 hours but not more than 48 hours, and then air dry.

#### 3.7.4 Dry-Pack Mortar

Prime surfaces prepared to receive repair material with a cement-based grout, an epoxy resin meeting the requirements of paragraph EPOXY RESIN, or a latex bonding agent meeting the requirements of paragraph LATEX BONDING COMPOUND. For cement-based grout, dampen the area with water prior to applying the grout. Place the mortar immediately after applying the bonding agent. Place mortar in approximately 10 mm (3/8-inch)3/8-inch lifts, compact each lift thoroughly with a blunt stick or hammer, and scratch the surface to promote bonding with subsequent lifts. Place consecutive layers continuously. Finish the top layer flush with the surrounding concrete and cure with saturated burlap or curing compound.

#### 3.7.5 Cement-Based Mortar or Concrete

\*\*\*\*\*

**NOTE: When formwork prevents application of bonding agents in a timely manner, new concrete should be applied directly to the existing concrete. Bonding agent should not be used when the concrete placement exceeds 3 inches in thickness.**

**Placement through a chimney at the top of the form is the preferred method of placement. Internal vibration should be used unless conditions prevent proper operation, in which case external vibration may be used.**

**Delete the options for form and pump placement and external vibration unless placement through a chimney is not possible and/or external vibration is**

permitted.

If the referenced specifications are not included in the project specification, applicable portions should be inserted here.

\*\*\*\*\*

[For vertical surfaces, construct forms with a chimney at top for placement and consolidation of concrete.] [Prime surfaces prepared to receive repair material, using a cement-based grout meeting the requirements of paragraph Cement-Based Grout, an epoxy resin meeting the requirements of paragraph EPOXY RESIN, or a latex bonding agent meeting the requirements of paragraph LATEX BONDING COMPOUND. For cement-based grout, dampen the area with water prior to applying the grout. Place the concrete immediately after applying the bonding agent.] Place, consolidate and finish concrete in accordance with applicable requirements of Section [03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [Section 03307 CONCRETE FOR MINOR STRUCTURES]. [Use form and pump method of placement where placement through a form chimney is not possible. Inspect concrete surfaces to ensure that no areas remain which would trap air between the new and existing concrete. Make forms grout tight and brace to withstand pumped concrete pressures. Provide vents to release air from within the forms as concrete is placed. Pump concrete into form using variable output pump of appropriate size.] [Use external vibration only where internal vibration is not possible. After the cavity is filled with concrete, install a pressure cap at the form chimney and apply pressure while the form is vibrated.] [Remove chimney projection after 24 hours.] Cure in accordance with Section [03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [Section 03307 CONCRETE FOR MINOR STRUCTURES].

### 3.7.6 Spalls at Joints and Cracks

\*\*\*\*\*

**NOTE:** If the referenced specification is not included in the project specification, applicable portions should be inserted here.

\*\*\*\*\*

For spalls to be repaired that are adjacent to joints and working cracks insert preformed joint filler to the working faces of the spall. Trim filler to fit shape of the working faces of joint or crack so repair material is prevented from bypassing filler. Where practicable, extend filler horizontally and vertically into joint or crack opening. Secure filler strip in place prior to and during placement of repair material. [Apply a bond breaker to working faces at keyed joints. Keep bond breaker off of concrete surface to be bonded.] After the repair material has completely cured, saw out the top 25 mm (inch) inch of the preformed joint filler and install joint sealant in accordance with Section 03151A EXPANSION, CONTRACTION AND CONSTRUCTION JOINTS IN CONCRETE FOR CIVIL WORKS.

### 3.7.7 Rapid-Hardening Repair Material

Mix and place material in accordance with the manufacturer's recommendations.

### 3.8 CRACK REPAIR

#### 3.8.1 Routing and Sealing

\*\*\*\*\*  
**NOTE: Surface sealing is not effective for active cracks. The minimum width of the prepared crack should be 6 mm1/4 inch.**  
\*\*\*\*\*

##### 3.8.1.1 Preparation

Clean cracks by saw cutting or routing to provide the proper profile for sealants. After the crack is sawcut or routed, clean the surfaces by sandblasting and/or waterjetting. Adjust the height, angle of inclination and the size of the nozzle as necessary to provide satisfactory results. Provide protective covers and barriers as required to prevent over-spray onto adjacent surfaces. Remove dust, dirt and loosely bonded materials resulting from cleaning. Ensure surfaces are dry before application of repair material. Apply bond breaker to the bottom of the routed or sawed crack. Where cracks extend through surfaces to be repaired, make the repair first and then tool, rout or saw the original line of the cracks into the repaired area.

##### 3.8.1.2 Rate of Progress

Limit the stages of crack sealing, which includes preparation and placement of bond breaker, so only that length of crack that can be sealed during the same workday is prepared.

##### 3.8.1.3 Time of Application

Seal prepared cracks immediately following the cleaning and drying process. Walls of the prepared crack shall be surface dry, and the atmospheric and concrete temperatures shall be above 10 degrees C (50 degrees F) 50 degrees F at the time of application of the sealant. Provide an approved temporary seal for open routed cracks that cannot be sealed under the conditions specified.

##### 3.8.1.4 Sealing

Do not apply sealant until the prepared cracks have been inspected and approved. [Place masking tape on the finish surface on one or both sides of a crack cavity to protect adjacent finish surfaces from primer or sealant smears. Remove masking tape within 10 minutes after crack has been filled and tooled.] Fill the cracks from the bottom up and tool the top surface as indicated or as recommended by the manufacturer. Apply sealant in accordance with ACI 504R.

##### 3.8.1.5 Preformed Compression Seals

Rout or cut the crack to the dimensions indicated. Install the seals with equipment that is capable of installing joint seals to the prescribed depth without cutting, nicking, twisting, or otherwise distorting or damaging the seal and with no more than five percent stretching of the seal. Cover the sides of the crack and, if necessary, the sides of the compression seal with a coating of lubricant, and install the seal to the depth indicated with joint installation equipment. Coat butt joints with liberal applications of lubricant.

### 3.8.2 Filling Cracks with Grout or Mortar

\*\*\*\*\*  
**NOTE: Filling cracks is not effective for active cracks. The prepared crack should be minimum 25 mm wide and 25 mm deep 1 inch wide and 1 inch deep.**  
\*\*\*\*\*

#### 3.8.2.1 Preparation

Clean cracks by saw cutting or routing to provide the proper profile for sealants. After the crack is sawcut or routed, clean the surfaces by sandblasting and/or waterjetting. Adjust the height, angle of inclination and the size of the nozzle as necessary to provide satisfactory results. Provide protective covers and barriers as required to prevent over-spray onto adjacent surfaces. Remove dust, dirt and loosely bonded materials resulting from cleaning. Dry surfaces to receive repair material as recommended by the grout or polymer manufacturer.

#### [3.8.2.2 Epoxy Grout

Apply epoxy grout at a thickness recommended by the manufacturer. Work grout into place and consolidate thoroughly so that contact surfaces are wetted by the grout. Finish surface of grout to the required texture. Do not feather edge epoxy grout onto adjacent surfaces.

#### ]3.8.2.3 Dry-Pack Mortar

Coat the surfaces of the crack with a slurry consisting of equal parts portland cement and fine sand mixed with water to a fluid paste consistency. Place the mortar immediately after applying the slurry. Place mortar in approximately 10 mm (3/8-inch) 3/8-inch lifts, compact each lift thoroughly with a blunt stick or hammer, and scratch the surface to promote bonding with subsequent lifts. Place consecutive layers continuously. Finish the top layer flush with the surrounding concrete and cure with saturated burlap or curing compound.

#### 3.8.2.4 Curing

\*\*\*\*\*  
**NOTE: If the referenced specifications are not included in the project specification, applicable portions should be inserted here.**  
\*\*\*\*\*

Cure epoxy materials in accordance with manufacturer's recommendations. Cure dry-pack mortar in accordance with Section [03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [Section 03307 CONCRETE FOR MINOR STRUCTURES].

### 3.8.3 Gravity Crack Filling

#### 3.8.3.1 Surface Preparation

Clean concrete surfaces that are contaminated with oil, grease or dirt prior to beginning surface preparation in the area. Use only detergents or proprietary cleaners formulated for removal of contaminants from concrete, and use them in accordance with the manufacturer's written instructions.

Do not use solvents for removal of oil or grease. Remove all traces of cleaning agents and contaminants from the surface and cracks. Perform final surface cleaning immediately prior to application of gravity filling in accordance with paragraph REMOVAL OF EXISTING CONCRETE, subparagraph Surface Preparation. Dry surfaces to receive repair material as recommended by the grout or polymer manufacturer.

#### 3.8.3.2 Applying Sealer

Apply high molecular weight methacrylate sealer to prepared surfaces in accordance with manufacturer's directions. Remove excess sealer, leaving no visible surface film. Fill cracks greater than 3 mm (1/8 inch) 1/8 inch wide in accordance with sealer manufacturer's recommendations. Apply a second treatment of sealer evenly over the cracks and remove excess sealer on the surface. Cover with light broadcast of a dry masonry sand. Distribute evenly over the surface at the rate recommended by the sealer manufacturer. After a curing period of 12 hours at 23 degrees C (73 degrees F) 73 degrees F, remove any loose sand by lightly brooming.

#### 3.8.4 Drilling and Plugging Cracks

\*\*\*\*\*  
NOTE: Drilling and plugging is applicable only for cracks which are reasonably straight and accessible from one end. The hole must be large enough to intercept the crack for its entire length. If load transfer is necessary, plug the hole with grout. If watertightness is necessary, but not load transfer, plug the hole with resilient material.  
\*\*\*\*\*

Drill a [50] [75] mm ([2] [3] inch) [2] [3] inch diameter hole centered on the crack. Clean the hole by flushing with water and compressed air prior to placing grout. Fill the hole with [portland cement grout consisting of one part portland cement, three parts fine aggregate, and sufficient water to permit placement in the hole] [bituminous joint sealant]. Place the grout in lifts to avoid pressure from the weight of the grout causing the grout to leak through the crack at lower levels.

#### 3.8.5 Injection Grouting Cracks

\*\*\*\*\*  
NOTE: Portland cement grout is normally used to seal wide cracks, but may be used to seal cracks as narrow as 0.05 mm 0.002 inch. Ultra-fine cement grout may be used to seal cracks of a smaller size. Epoxy resin is normally used for cracks between 0.13 and 6.3 mm 0.005 and 0.25 inch wide. Polyurethane is normally used for cracks greater than 0.13 mm 0.005 inch wide.

The interval between valves should be greater than the thickness of the member being repaired.

\*\*\*\*\*

##### 3.8.5.1 Preparation

Clean each crack of dust, dirt, loose concrete and unsound material by vacuuming or flushing with water and allowing to dry to remove free water

in the crack. Insert a valve at both ends of each crack, at the junction of two cracks, and along the length of each crack at [400 to 500] [ ] mm ([16 to 20] [ ] inch) [16 to 20] [ ] inch intervals. Fill crack between valves with crack surface sealer.

#### 3.8.5.2 Injection

After crack surface sealer has hardened and cured, pump [epoxy injection adhesive] [polyurethane injection adhesive] [ultra-fine cement grout] into valve at one end of crack. For vertical surfaces start at lowest valve and work upwards. As crack sealer appears at next valve, pinch pumping valve closed and move to next valve and commence pumping. Continue procedure until other end of crack is reached. Avoid delays in pumping operation. After crack sealer has hardened and cured grind valves off flush with concrete surface. Coat areas of valves with crack surface sealer and allow to harden and cure.

#### 3.8.6 Crack Repair with Additional Reinforcement

\*\*\*\*\*  
**NOTE: Repairing cracks with additional reinforcement is used to stabilize active cracks to permit sealing and to reestablish tensile strength across cracks.**  
\*\*\*\*\*

Stabilize cracks by anchoring as shown on the drawings.

##### 3.8.6.1 Stitching

\*\*\*\*\*  
**NOTE: When the crack must be made watertight, one of the methods for sealing or repairing the crack should be used prior to stabilizing the crack.**  
\*\*\*\*\*

Drill holes of appropriate size and depth for grouting the stitching staples and at the spacing shown on the drawings. Reduce spacing at ends of crack. Vary the orientation and length of stitching staples to avoid applying tension to a single plane within the concrete. If the end of the crack within the structure can be determined, drill a hole at the end of the crack to relieve stress concentration. Clean the holes by flushing with water and compressed air prior to placing grout. Dry surfaces to receive grout as recommended by the grout manufacturer. Partially fill the holes with epoxy resin grout and insert the stitching staples into the holes. Place additional grout and consolidate it by rodding until remainder of the hole is filled. Place grout with the equipment capable of supplying, mixing, and placing the grout in the holes in a manner that will ensure complete filling of the holes and elimination of voids, air pockets, and water. After placing, allow the grout to set for a period of not less than 24 hours, and protect the stitching staples against damage. Remove and reset any stitching staples which are disturbed during curing.

##### 3.8.6.2 Conventional Reinforcement

Drill holes, of a size appropriate for the reinforcement and grout used, at right angle to the crack plane, and extending a minimum of 0.5 m (18 inches) 18 inches on each side of the crack. Space holes as indicated on the drawings. Clean the holes by flushing with water and compressed air prior to placing grout. Insure that holes are dry before placing grout. Inject

epoxy grout at a pressure sufficient to distribute the grout in the hole but not great enough to cause additional damage to the concrete. Insert reinforcing bar so that it extends at least 0.5 m (18 inches) 18 inches on each side of the crack. [After grout has set, seal the crack by injection grouting.]

#### 3.8.6.3 Internal Prestressing Reinforcement

\*\*\*\*\*  
NOTE: Where both sides of the structure are accessible, the prestressing steel may extend from an anchor plate on one side with the stressing head at the other side with the hole grouted after stressing. When only one side is exposed, the prestressing steel is grouted in place and stressed.  
\*\*\*\*\*

Drill holes of the size and depth [shown on the drawings] [recommended by the grout manufacturer] and at the location shown. Cut recess for [anchor plate and] stressing head in the concrete as shown. Clean the hole by flushing with water and compressed air prior to placing grout. Dry holes as recommended by the grout manufacturer.

Place [cement-based] [polymer-modified] [epoxy] grout pads [in the recess(es)] [on the face of the wall] as shown on the drawings and install the [anchor plate,] stressing plate. [Install the threaded bar and nuts and tighten until snug until the grout pads have set.] [Insert the required resin cartridges into the drilled hole, taking care not to rupture the skin. Use rapid setting cartridges in the anchor zone of the bar and slower setting cartridges in the stressing zone. To avoid premature rupture of the resin cartridges it may be necessary to place them in a thin-walled tube and insert the tube in the drilled hole to the specified depth. Withdraw the tube carefully prior to insertion of the threaded bar. Rotate the threaded bolt through the cartridges to end of hole in accordance with the cartridge manufacturer's recommendations in order to rupture the skin and mix the resin. Use method, rotation time, and speed of rotation as recommended by the manufacturer and approved by the Contracting Officer.] No permanent installation of threaded bars will be permitted until it is demonstrated to the satisfaction of the Contracting Officer that the anchors can be properly installed. Stress the threaded bar to [ ] KPa [ ] psi using equipment recommended by the bar manufacturer. This load is the final effective (residual) load per bar after all losses have occurred. Determine the stress load by the measurement of the threaded bar elongation and by checking pressure of the hydraulic jack. Correlation shall be within 5 percent of agreement. After stressing the threaded bar, cut the bar and fill the recess with dry-pack mortar.

#### 3.8.6.4 External Stressing

Install anchorages as shown on the drawings. Install prestressing bars and stress in increments in accordance with the procedure shown on the drawings to avoid eccentric loads.

### 3.9 CLEANING AND RESEALING JOINTS

\*\*\*\*\*  
NOTE: The drawings must show typical details for the joint sealing.  
\*\*\*\*\*

\*\*\*\*\*

### 3.9.1 Preparation of Joints

Immediately before the installation of the sealant, thoroughly clean the joints to remove all laitance, loose or deteriorated filler, and old sealant from the sides and upper edges of the joint space to be sealed. Clean joints to sufficient depth to remove all loose material and debris and to permit placement of the backup material and sealant as shown on the drawings. Concrete surfaces shall be dry at the time of sealant application.

#### 3.9.1.1 Existing Sealant Removal

Cut the in-place sealant and expansion joint material loose from both joint faces and to the minimum depth required to permit placement of the back-up material and sealant depth equal to one half the width of joint or as recommended by the sealant manufacturer, unless otherwise shown on the drawings, using the waterjetting equipment as specified in paragraph EQUIPMENT. Removal may be efficiently accomplished using high pressure water jet with abrasive added. Prior to further cleaning operations, remove all loose old sealant remaining in the joint opening by blowing with compressed air. Chipping, spalling, or otherwise damaging the concrete will not be permitted. [ For compression seals, sawcut the joint to the width and depth shown on the drawings.]

#### 3.9.1.2 Sandblasting and Waterjetting

Sandblast or waterjet clean the newly exposed concrete joint faces to at least the depth of the back-up material and sealant and the concrete surfaces extending a minimum of 13 mm (1/2 inch) 1/2 inch from the joint edges. Use a multiple-pass technique until the surfaces are free of dust, dirt, filler, old sealant residue, or any foreign debris that might prevent the bonding of the sealant to the concrete. After final cleaning and immediately prior to sealing, blow the joints out with compressed air to ensure that the joints are completely free of debris and water.

#### 3.9.1.3 Rate of Progress of Joint Preparation

Limit the joint preparation which includes sandblasting or waterjetting, air pressure cleaning and placing of the back-up material to only that lineal footage that can be sealed during the same day.

### 3.9.2 Installation of Sealant

Store and apply the primer and sealant in accordance with the manufacturer's written safety instructions and precautions and using manufacturer recommended procedures.

#### 3.9.2.1 Time of Application

Seal joints immediately following final cleaning of the joint sidewalls and following the placement of the bond breaker and back-up material. Reclean open joints that cannot be sealed under the conditions specified, or when rain interrupts sealing operations, and allowed to dry prior to installing the sealant.

### 3.9.2.2 Back-Up Material and Bond Breaker

Plug or seal off the lower portion of the joint opening using a back-up material to prevent the entrance of the sealant below the specified depth. Take care to ensure that the backup material is placed at the specified depth and is not stretched or twisted during installation.

### 3.9.2.3 Sealing Joints

Perform a final cleaning with compressed air immediately preceding, but not more than 50 feet ahead of the sealing operations. The joint surfaces shall be dry at the time the sealant is installed. Apply primer evenly to the joint faces in accordance with the manufacturer's instructions before placement of the back-up material. Place bond breaker material after the primer is applied. Fill the joints from the bottom up to the [bottom of the beveled edge][surface of the concrete]. Depth of sealant shall be one-half the joint width or as recommended by the sealant manufacturer. [Commence sealing of joints at the bottom of the slope or wall and continue up the concrete slope or wall]. Remove excess or spilled sealant from the concrete surface by approved methods and discard. Install the sealant using a gun cartridge in such a manner as to prevent the formation of voids and entrapped air. Do not install sealant material using gravity methods or pouring pots. Check joints frequently to ensure that the newly installed sealant is cured to a tack-free condition within the time specified. Tool the sealant concave immediately after application.

### [3.9.3 Installation of Preformed Compression Seals

Install the joint seals with equipment capable of installing joint seals to the prescribed depth without cutting, nicking, twisting, or otherwise distorting or damaging the seal and with no more than five percent stretching of the seal. Cover the sides of the joint and, if necessary, the sides of the compression seal, with a coating of lubricant, and install the seal to the depth indicated with joint installation equipment. Coat butt joints with liberal applications of lubricant.

## ]3.10 OVERLAYS

\*\*\*\*\*  
NOTE: If the referenced specifications are not  
included in the project specification, applicable  
portions should be inserted here.  
\*\*\*\*\*

### 3.10.1 Cement-Based Concrete Overlay

\*\*\*\*\*  
NOTE: When overlaying mass concrete with a  
relatively thin layer of cement-based concrete or  
where movement in the base concrete may occur, a  
bond breaker should be placed to prevent shrinkage  
and restraint cracking of the overlay due to  
constraint of the overlay.  
\*\*\*\*\*

Install anchors and reinforcement [and place bond breaker] as shown on the drawings. Place, consolidate, finish and cure concrete in accordance with the applicable requirements of Section [03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [Section

03307 CONCRETE FOR MINOR STRUCTURES], except only moist curing will be permitted.

### 3.10.2 Polymer-Modified Concrete Overlay

Mixing, placing and curing polymer-modified concrete shall be in accordance with ACI 548.4 and the following requirements. Mix and place polymer-modified concrete only within the temperature limits recommended by the polymer manufacturer. Prime the surface with a slurry of polymer-modified mortar of the same composition as the polymer-modified concrete except for coarse aggregate. Place polymer-modified concrete before the prime coat dries. Place polymer-modified concrete within 15 minutes of addition of polymer. Moist cure overlay for not less than 24 hours nor more than 48 hours, then air dry.

### 3.10.3 Joints

Construct expansion and contraction joints in concrete overlay at the locations shown. Maintain alignment of control joints within 1/4-inch, to either side, of the required joint alignment. Construct expansion and contraction joints at the locations shown and in accordance with Section 03150A EXPANSION JOINTS, CONTRACTION JOINTS, AND WATERSTOPS. Construct expansion joints at existing joints using preformed expansion joint material of the thickness shown and extending the full depth of the overlay. Construct control joints by tooling the plastic concrete, then sawcutting at the appropriate time. Saw control joints to a minimum [depth of [ ] mm ([ ] inch(es)) [ ] inch(es)] [of 25 percent of the thickness of the slab]. Maintain an ample supply of saw blades on the job before concrete placement is started, and have at least one standby sawing unit in good working order available at the jobsite at all times during the sawing operations. Begin sawcutting as soon as it is possible to saw the concrete without damaging adjacent concrete. Inspect the faces of joints during sawcutting for undercutting or washing of the concrete due to early sawing.

Complete sawcutting within 16 hours of concrete placement. Continue sawcutting regardless of weather conditions. Delay sawing if undercutting is sufficiently deep to cause structural weakness or excessive roughness in the joint or chipping, tearing, or spalling of the concrete occurs at the surface. Discontinue sawing when a crack develops ahead of the saw cut. Immediately after the joint is sawed, flush the saw cut and adjacent concrete surface thoroughly with water until all residue from sawing is removed from the joint. Control and dispose of waste water from sawcutting and cleanup in accordance with Section 01355A ENVIRONMENTAL PROTECTION.

## 3.11 REFACING VERTICAL SURFACES

### 3.11.1 Cement-Based Concrete

\*\*\*\*\*  
NOTE: Where there is concern about placement of high lifts of refacing concrete, use the optional text concerning lift height and placement time. The permissible height of individual lifts and time limit between placing lifts should be determined by the form design to prevent overloading of the forms and to maintain required construction tolerances.  
\*\*\*\*\*

Install anchors and reinforcement as shown on the drawings. Place,

consolidate, finish and cure concrete in accordance with the applicable requirements of Section [03300A CAST-IN-PLACE STRUCTURAL CONCRETE] [03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [Section 03307 CONCRETE FOR MINOR STRUCTURES] [Section 03371 SHOTCRETE]. Do not place concrete when, in the opinion of the Contracting Officer, weather conditions prevent proper placement, consolidation, and curing. [In order to prevent overloading of forms, place concrete systematically in lifts not to exceed [1.3] [ ] m ( ) [4] [ ] feet, across the complete placement area. Place the next lift no sooner than [4] [ ] hours from the time that the first lift is placed.] Deposit concrete as close as possible to its final position in the forms, and in so depositing do not permit a vertical drop greater than 1.5 m five feet except where suitable equipment is provided to prevent segregation and where specifically authorized. Regulate depositing of the concrete so that it may be effectively consolidated in horizontal layers 600 mm (2.0 feet) 2.0 feet or less in thickness with a minimum of lateral movement. Deposit in each location only that amount of concrete that can be readily and thoroughly consolidated. Thoroughly distribute and consolidate concrete around embedded items, assuring that no air pockets occur behind vertical items. Provide sufficient placing capacity so that concrete can be kept plastic and free of cold joints while concrete is being placed.

#### 3.11.2 Preplaced-Aggregate Concrete

Install anchors and reinforcement as shown on the drawings. Place, consolidate, finish and cure concrete in accordance with the applicable requirements of Section 03372 PREPLACED-AGGREGATE CONCRETE.

#### 3.11.3 Precast Concrete Units

Prepare base and install anchors as shown on the drawings. Fabricate precast concrete units in accordance with Section 03410 PRECAST [PRESTRESSED] STRUCTURAL CONCRETE. Cure by steam at atmospheric pressure only.

##### 3.11.3.1 Erection of Precast Units

Erect precast units in accordance with the details shown on the approved shop drawings and erection plan for precast units, alignment plan for precast units, approved revisions to the approved plans resulting from the precast unit assembly test, and in accordance with Section 03410 PRECAST [PRESTRESSED] STRUCTURAL CONCRETE. Install equipment required by other trades as the work progresses if required by design. Do not field-cut openings in the units unless recommended by the manufacturer and approved by the Contracting Officer. Ensure that all bearing surfaces are level and free from irregularities. Install precast units at right angles to bearing surfaces, drawn up tight without forcing or distortion, and with sides plumb. Set precast units true to alignment, level and plumb, with joints properly spaced and aligned both vertically and horizontally. Place shims as required as units are erected to maintain correct alignment. Install [elastomeric expansion joint filler material], back-up material, and neoprene bearing pads as shown on the contract drawings. Bond elastomeric filler material and neoprene bearing pads to the precast units using adhesive as recommended by the manufacturers. After erection, fill pickup points, inserts, and similar items and finish with dry-pack mortar to match adjacent areas. Erection tolerances shall be in accordance with the requirements of PCI MNL-116.

### 3.11.3.2 Temporary Unit Supports

Install temporary unit supports to align precast units and support the units during infill concrete placement. Place the temporary unit supports well in advance of any scheduled infill concrete placement. Secure temporary unit supports prior to infill concrete placements and keep in place after infill placement for at least 24 hours, unless appropriate calculation and break histories verify that the infill concrete has reached a strength of 500 psi sooner. Do not remove temporary unit supports in less than 12 hours.

### 3.11.3.3 Installation of Neoprene Bearing Pads and Joint Filler Material

Install neoprene bearing pads in accordance with the details shown on the contract drawings. Install expansion joint filler material in horizontal and vertical joints between all precast concrete units in accordance with the details shown on the contract drawings. Install neoprene bearing pads accurately to the locations shown. Apply adhesive to neoprene bearing pads and expansion joint filler material to bond these materials to the concrete surfaces of the precast units. Allow adhesive to cure for an appropriate time.

### 3.11.3.4 Placement of Infill Concrete

After precast units have been erected, plumbed, leveled, and secured with the temporary unit supports, place infill concrete to fill the void behind the precast units as shown. Place infill concrete behind the [bottom and middle] units in lifts of the heights shown on the drawings. Do not place consecutive lifts in less than the time indicated in the approved erection plan. Ensure that all formwork at joints and open ends are leak-tight, and sufficiently rigid to withstand the pressures encountered during the infill concrete placement. Block elastomeric filler material, neoprene bearing pads, expansion joint material, back-up material, and shims sufficiently to prevent movement during placement of infill concrete. Begin curing of infill concrete as soon as possible after completion of the placement and continue until the next lift of precast units is installed. Cure the exposed infill concrete in accordance with [Section 03301A CAST-IN-PLACE STRUCTURAL CONCRETE FOR CIVIL WORKS] [Section 03307 CONCRETE FOR MINOR STRUCTURES].

### 3.11.4 Shotcrete

Install anchors and reinforcement as shown on the drawings. Place, finish and cure shotcrete in accordance with the applicable requirements of Section 03371 SHOTCRETE.

## 3.12 TESTS AND INSPECTIONS

### 3.12.1 General

The individuals who sample and test repair materials as required in this specification shall have demonstrated a knowledge and ability to perform the necessary test procedures equivalent to the ACI minimum guidelines for certification of Concrete Field Testing Technicians, Grade I.

### 3.12.2 Preparations for Placing

Inspect prepared surfaces, forms, and embedded items in sufficient time prior to each placement of repair material to certify that the surfaces are

ready to receive the repair material.

### 3.12.3 Grouted Dowels and Anchors

Test the first three dowels or anchors of each type and a minimum of three dowels or anchors per 1,000 dowels or anchors in accordance with ASTM E 488.

Use incremental loading for tensile test to 75% of the yield strength of the dowel or anchor. Consider anchors to have failed if displacement exceeds 2.5 mm (0.1 inch) or if any of the failure modes of paragraph 12.2 occur.

### 3.12.4 Epoxy Mortar

#### 3.12.4.1 Sampling

As soon as epoxy resin and aggregate materials are available for sampling, obtain by random selection a sample of each batch. Clearly identify samples by designated name, specification number, batch number, project contract number, intended use and quantity involved.

#### 3.12.4.2 Testing

\*\*\*\*\*  
**NOTE: For projects requiring large amounts of epoxy repairs use the bracketed sentences.**  
\*\*\*\*\*

At the discretion of the Contracting Officer, samples provided may be tested by the Government for verification. [Test samples by an approved laboratory. If a sample fails to meet specification requirements after two tests, replace the batch represented by the samples tested and retest. Test aggregates in accordance with ASTM C 117 and ASTM C 136.]

#### 3.12.4.3 Inspection

\*\*\*\*\*  
**NOTE: Use this paragraph for projects having large repaired surface areas.**  
\*\*\*\*\*

Check each repaired area for cracks, spalls, popouts and loss of bond between repaired area and surrounding concrete. Check each repaired area for voids by tapping with a hammer or steel rod and listening for dull or hollow sounds. Immediately repair defects.

### 3.12.5 [Cement-Based ] [or] [Polymer-Modified] Concrete and Mortar

#### 3.12.5.1 Air Content

Check air content at least [once] [twice] during each shift that concrete is placed [for each type and class of repair material required]. Obtain samples in accordance with ASTM C 172 and test in accordance with ASTM C 231.

#### 3.12.5.2 Slump

Check slump [once] [twice] during each shift that concrete is produced [for each type and class of repair material required]. Obtain samples in accordance with ASTM C 172 and test in accordance with ASTM C 143/C 143M.

#### 3.12.5.3 Consolidation and Protection

Ensure that the repair material is properly consolidated, finished, protected, and cured.

#### 3.12.5.4 Compression Tests

Prepare compression test cubes in accordance with ASTM C 31/C 31M and cure at the site under the same conditions as the repair. Test in accordance with ASTM C 109/C 109M.

#### 3.12.5.5 Curing

a. Moist-Curing Inspections - At least once each shift, and once per day on nonwork days, inspect all areas subject to moist curing. Note and record the surface moisture condition.

b. Membrane-Curing Inspection - Do not apply curing compound until the Contractor's authorized representative has verified that the compound is properly mixed and ready for spraying. At the end of each operation, estimate the quantity of compound used by measurement of the container and the area of repair material surface covered and compute the rate of coverage in square meters per liter square feet per gallon. Note whether or not coverage is uniform.

c. Sheet-Curing Inspection - At least once each shift and once per day on nonwork days, inspect all areas being cured using material sheets. Note and record the condition of the covering and the tightness of the laps and tapes.

#### 3.12.6 Aggregates

Test gradation in accordance with ASTM C 136 and ASTM C 117. Determine the percent passing the No. 200 sieve by washing in accordance with ASTM C 117.

#### 3.12.7 Action Required

##### 3.12.7.1 Placing

The placing foreman shall not permit placing to begin until he has verified that appropriate placement, consolidation and finishing equipment, which are in working order and have competent operators, are available.

##### 3.12.7.2 Grouted Anchors and Dowels

Test a minimum of two adjacent anchors or dowels for each anchor or dowel that fails.

##### 3.12.7.3 Air Content

Whenever a test result is outside the specification limits, do not deliver the concrete to the forms and adjust the dosage of the air-entrainment admixture.

##### 3.12.7.4 Slump

Whenever a test result is outside the specification limits, do not deliver the concrete to the forms and an adjustment should be made in the batch weights of water and fine aggregate. The adjustments are to be made so

that the water-cementitious materials ratio does not exceed that specified in the submitted concrete mixture proportion.

#### 3.12.7.5 Curing

- a. Moist-Curing Corrective Action - When a daily inspection report lists an area of inadequate curing, take immediate corrective action, and extend the required curing period for such areas by one (1) day.
- b. Membrane-Curing Corrective Action - When the coverage rate of the curing compound is less than that specified or when the coverage is not uniform, spray the entire surface.
- c. Sheet-Curing Corrective Action - When a daily inspection report lists any tears, holes, or laps or joints that are not completely closed, promptly repair the tears and holes or replace the sheets, close the joints, and extend the required curing period for those areas by one (1) day.

#### 3.12.8 Final Inspection

Following completion of the work, inspect surfaces for damage, staining, and other distresses. Inspect [repairs] [overlays] [and] [refacing] for cracking, crazing, delamination, unsoundness, staining and other defects. Inspect the finish and surface tolerances of the repairs to verify that all requirements have been met. Repair all surfaces exhibiting defects as directed at no cost to the Government when defects are due to contractor workmanship or procedures.

#### 3.12.9 Reports

Report the results of all tests and inspections conducted at the project site informally at the end of each shift and in writing weekly and deliver within 3 days after the end of each weekly reporting period. See Section 01451A CONTRACTOR QUALITY CONTROL.

#### 3.12.10 Manufacturer Field Service

Provide the services of a manufacturer's technical representative experienced in mixture proportioning and placement procedures for the following materials:

- a. Epoxy mortar or concrete
- b. Polymer-modified mortar or concrete
- b. Epoxy injection grout
- d. High molecular weight methacrylate sealer
- e. Concrete containing High-Range Water Reducers (HRWRA)
- f. Concrete containing Silica Fume

Provide, at no additional cost to the Government, the services of the manufacturer's experienced technical representative during mixture proportioning, planning and production. The manufacturer's representative shall be available for consultation by both the Contractor and the Contracting officer during mixture proportioning, planning, and production of the materials and shall be on-site immediately prior to and during at least the first placement of the material, and at other times if directed.

### 3.13 CLEAN UP

Clean all surfaces of concrete and adjacent facilities which are stained by dirt, oil, grease, fuel, or other byproducts that are created by the construction operations with detergent and pressure wash. Dispose of debris in accordance with Section 01572 CONSTRUCTION AND DEMOLITION WASTE MANAGEMENT[ and Section 01355A ENVIRONMENTAL PROTECTION].

### 3.14 DUST CONTROL

Control dust resulting from demolition to prevent the spread of dust and avoid creation of a nuisance in the surrounding area. Do not use water when it will result in, or create, hazardous or objectionable conditions such as ice, flooding, or pollution.

### 3.15 PROTECTION PRIOR TO ACCEPTANCE

Do not permit vehicular or heavy equipment traffic on the repair surfaces [during the curing period] [until [7] [ ] days after completion of the repair]. Permit light local traffic on the concrete surfaces at the end of the curing period, if approved by the Contracting Officer. Where shelter or other protective measures are provided for repair during inclement weather, maintain such protective measures until the repair material has cured and discontinuance of the measures is authorized.

### 3.16 SCHEDULES

#### Concrete Removal Methods

Feature	Removal Method										
	A	B	C	D	E	F	G	H	I	J	K
Horizontal Surface		X	X	X	X	X	X	X	X		
Vertical Surfaces	X	X		X	X	X	X	X	X	X	X
Slab-on-grade			X			X	X			X	X

- A Controlled Blasting
- B Abrasive Water Jet Cutting
- C Diamond Blade Cutting
- D Diamond Wire Cutting
- E Stitch Cutting
- F Boom Mounted Breaker
- G Hand Held Breaker
- H Hydromilling
- I Rotary Head Milling
- J Presplitting (Expansive Agent)
- K Mechanical Presplitting

## Repair Material

Feature	Repair Material								
	A	B	C	D	E	F	G	H	I
Horizontal Surfaces (repair)		X		X	X	X	X		
Horizontal Surfaces (Overlay)	X		X		X		X		
Vertical Surfaces (repair)	X	X	X	X	X	X		X	X
Vertical Surfaces (Refacing)	X		X		X		X	X	X

- A Cement-Based Concrete
- B Cement-Based Mortar
- C Precast Portland Cement Concrete
- D Polymer-Modified Mortar
- E Polymer-Modified Concrete
- F Epoxy Mortar
- G Epoxy Concrete
- H Preplaced-Aggregate Concrete
- I Shotcrete

## Crack Repair

Type of Crack	Repair Method					
	A	B	C	D	E	F
Open Crack on Surface	X	X				
Structural Crack (Inactive)	X	X		X	X	
Structural Crack (Active)			X			X

- A Clean and Seal
- B Clean and Fill
- C Drill and Plug
- D Injection
- E Stitching
- F Mechanical Anchoring

-- End of Section --