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USACE / NAVFAC / AFCEA / NASA UFGS-35 31 19.20 (April 2006)  
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Preparing Activity: USACE Replacing without change  
UFGS-02382 (September 2003)

## UNIFIED FACILITIES GUIDE SPECIFICATIONS

References are in agreement with UML dated 1 April 2006

Latest change indicated by CHG tags

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#### SECTION 35 31 19.20

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04/06

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SECTION 35 31 19.20

ARTICULATING CONCRETE BLOCK REVETMENTS  
04/06

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NOTE: This guide specification covers the requirements for commercially available concrete block products for revetments.

Comments and suggestions on this guide specification are welcome and should be directed to the technical proponent of the specification. A listing of technical proponents, including their organization designation and telephone number, is on the Internet.

Recommended changes to a UFGS should be submitted as a Criteria Change Request (CCR).

Use of electronic communication is encouraged.

Brackets are used in the text to indicate designer choices or locations where text must be supplied by the designer.

This guide specification includes tailoring options for interlocking blocks, cabled systems, AASHTO geotextile, and site specific geotextile. Selection or deselection of a tailoring option will include or exclude that option in the section, but editing the resulting section to fit the project is still required.

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PART 1 GENERAL

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NOTE: PART 2 PRODUCTS is based on commercial items, and does not address field casting of blocks or manufacturing custom blocks.

This guide specifications assumes the ACB is Government designed for issues such as hydraulic stability and geotextile filters. Maximum

flexibility is desirable for Contractor product selection, installation sequence, construction equipment, and block orientation.

Notes before paragraphs are provided to present assumptions in preparation of the guide specification, make suggestions for conditions that warrant project revisions, and provide background technical information or references for further information. They should be reviewed prior to revising wording for use in project specifications.

The drawings should show appropriate details for toe key-in, anchor trenches, revetment termination, transition to riprap, anchors, etc.

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## 1.1 REFERENCES

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NOTE: This paragraph is used to list the publications cited in the text of the guide specification. The publications are referred to in the text by basic designation only and listed in this paragraph by organization, designation, date, and title.

Use the Reference Wizard's Check Reference feature when you add a RID outside of the Section's Reference Article to automatically place the reference in the Reference Article. Also use the Reference Wizard's Check Reference feature to update the issue dates.

References not used in the text will automatically be deleted from this section of the project specification when you choose to reconcile references in the publish print process.

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The publications listed below form a part of this specification to the extent referenced. The publications are referred to within the text by the basic designation only.

AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS  
(AASHTO)

AASHTO M 288 (2005) Geotextile Specifications for Highway Applications

ASTM INTERNATIONAL (ASTM)

ASTM C 1262 (1998e1) Evaluating the Freeze-Thaw Durability of Manufactured Concrete Masonry Units and Related Concrete Units

ASTM C 140 (2005) Sampling and Testing Concrete Masonry Units and Related Units

ASTM C 42/C 42M	(2004) Obtaining and Testing Drilled Cores and Sawed Beams of Concrete
ASTM D 4355	(2002) Deterioration of Geotextiles from Exposure to Light, Moisture and Heat in a Xenon-Arc Type Apparatus
ASTM D 4491	(1999; R 2004) Water Permeability of Geotextiles by Permittivity
ASTM D 4533	(2004) Trapezoid Tearing Strength of Geotextiles
ASTM D 4632	(1991; R 2003) Grab Breaking Load and Elongation of Geotextiles
ASTM D 4751	(2004) Determining Apparent Opening Size of a Geotextile
ASTM D 4833	(2000e1) Index Puncture Resistance of Geotextiles, Geomembranes, and Related Products
ASTM D 4873	(2002) Identification, Storage, and Handling of Geosynthetic Rolls and Samples

#### U.S. FEDERAL HIGHWAY ADMINISTRATION (FHWA)

FHWA RD-89-199	(1989) Hydraulic Stability of Articulated Concrete Block Revetment Systems During Overtopping Flow
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## 1.2 DEFINITIONS

### 1.2.1 Articulating Concrete Block (ACB) Revetment System

A matrix of interconnected concrete block units for erosion protection. Units are connected by geometric interlock and/or cables, geotextiles, or geogrids, and typically include a geotextile underlayment for subsoil retention.

### 1.2.2 Blocks

Articulating concrete block revetment units will be referred to as blocks.

### 1.2.3 Interlocking Blocks

Each pair of abutting blocks shall have interlocking keys that limit lateral expansion. The key and keyhole shall have an interference fit such that the joint movement has a minimum aperture at closure, and a maximum aperture when pulled apart. The joint freeplay shall allow articulation of each individual block.

### 1.2.4 Freeplay

Freeplay shall be the maximum lateral joint movement for interlocking blocks (difference between maximum and minimum aperture).

### 1.3 SUBMITTALS

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NOTE: Review submittal description (SD) definitions in Section 01 33 00 SUBMITTAL PROCEDURES and edit the following list to reflect only the submittals required for the project. Submittals should be kept to the minimum required for adequate quality control.

A "G" following a submittal item indicates that the submittal requires Government approval. Some submittals are already marked with a "G". Only delete an existing "G" if the submittal item is not complex and can be reviewed through the Contractor's Quality Control system. Only add a "G" if the submittal is sufficiently important or complex in context of the project.

For submittals requiring Government approval on Army projects, a code of up to three characters within the submittal tags may be used following the "G" designation to indicate the approving authority. Codes for Army projects using the Resident Management System (RMS) are: "AE" for Architect-Engineer; "DO" for District Office (Engineering Division or other organization in the District Office); "AO" for Area Office; "RO" for Resident Office; and "PO" for Project Office. Codes following the "G" typically are not used for Navy, Air Force, and NASA projects.

Choose the first bracketed item for Navy, Air Force and NASA projects, or choose the second bracketed item for Army projects.

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Government approval is required for submittals with a "G" designation; submittals not having a "G" designation are for [Contractor Quality Control approval.] [information only. When used, a designation following the "G" designation identifies the office that will review the submittal for the Government.] The following shall be submitted in accordance with Section 01 33 00 SUBMITTAL PROCEDURES:

#### SD-02 Shop Drawings

Block Installation[; G][; G, [\_\_\_\_]]  
Geotextile Installation[; G][; G, [\_\_\_\_]]

Drawings shall be submitted that show details of the ACB and Geotextile Installation. The details shall show the block layout patterns in relation to the feature alignment, anticipated locations of cast-in-place concrete joints, mattress junction details, soil anchors, and proposed installation methods for void filling materials.

#### SD-03 Product Data

Articulating Concrete Block[; G][; G, [\_\_\_\_]]  
Geotextile[; G][; G, [\_\_\_\_]]

Descriptive technical data shall be submitted on the blocks, cables, cable fittings, soil anchors, and geotextile. The submittal shall include all material properties specified under paragraph PRODUCTS. Catalog cuts, technical data sheets, or test data shall be submitted showing that the products meet the specifications. The submittal shall also include a copy of any standard manufacturer's warranties for the products.

Anchors[; G][; G, [\_\_\_\_\_]]

The Contractor shall submit calculations for the anchor pullout capacity. Tabulated manufacturer's data is acceptable, if the embedment soil conditions are applicable to the project site.

#### SD-04 Samples

##### Articulating Concrete Block

At the same time as the ACB and Geotextile Data submittal, the Contractor shall submit two samples of the proposed block. The samples shall be typical of the size, texture, color, and finish. If the Contracting Officer is familiar with the product, this submittal may be waived.

#### SD-06 Test Reports

##### Flume Test

At the same time as the ACB and Geotextile Data submittal, the Contractor shall submit a report of testing for the ACB in substantial conformance with FHWA RD-89-199. The report shall clearly state if the critical shear stress associated with the stability threshold of the ACB system was derived from laboratory testing that included a sub-block drainage layer as a component of the tested system.

### 1.4 SCHEDULE

To limit ultraviolet light exposure of the geotextile, the blocks shall generally be placed within 7 days after placing the geotextile, and the void filler shall generally be placed within 14 days after placing the geotextile.

### 1.5 DELIVERY, STORAGE AND HANDLING

The Contractor shall check products upon delivery to assure that the proper material has been received and is undamaged. For geosynthetics, the guidelines presented in ASTM D 4873 shall be followed.

#### 1.5.1 Blocks

All blocks shall be sound and free of defects that would interfere with proper placement or that would impair the strength or longevity of the installation. Blocks with the following defects shall be discarded:

- a. Blocks with broken appendages.
- b. Blocks with chips larger than 50 mm 2 inches in any dimension.
- c. Blocks with cracks wider than 0.5 mm 0.02 inches and longer than

33% of the nominal height.

Minor cracks incidental to the usual method of manufacture, or chipping that results from customary methods of handling in shipping, delivery and placement will not be deemed grounds for rejection. Blocks shall be stored in a suitable location away from mud, paint, wet cement, and other contamination or disturbance.

#### 1.5.2 Geotextiles

##### 1.5.2.1 Labeling

Each roll shall be labeled with the manufacturer's name, product identification, roll dimensions, lot number, and date manufactured.

##### 1.5.2.2 Handling

Geosynthetic rolls shall be handled and unloaded by hand, or with load carrying straps, a fork lift with a stinger bar, or an axial bar assembly. Geosynthetic rolls shall not be dragged, lifted by one end, lifted by cables or chains, or dropped to the ground.

##### 1.5.2.3 Storage

Geotextiles shall be protected from cement, paint, excessive mud, chemicals, sparks and flames, temperatures in excess of 70 degrees C 160 degrees F, and any other environmental condition that may degrade the physical properties. If stored outdoors, the rolls shall be elevated from the ground surface. Geotextiles shall be protected with an opaque waterproof cover. Geotextiles shall be delivered to the site in a dry and undamaged condition.

#### 1.6 MEASUREMENT AND PAYMENT

Measurement of ACB revetment for payment will be made on the basis of the face area. The pay lines of ACB revetment will be neat lines taken off the approved shop drawings; and will include embedded blocks and anchor trenches. Work includes incidental grading and preparatory work, furnishing and installing the geotextile and ACB, filling the voids, securing cable fasteners, installing soil anchors, and seeding (where specified). Engineering services and product testing shall be incidental, if required. Placing cast-in-place concrete joints and cutting blocks shall be incidental, if required. Payment will be made at the respective unit price per square meter (SM) foot (SF) listed on the Bidding Schedule. Payment will be full compensation for all material, labor and equipment to complete the work.

#### PART 2 PRODUCTS

##### 2.1 ARTICULATING CONCRETE BLOCK

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**NOTE:** Hand placed (interlocking) ACB typically provides a neater, more pleasing appearance in parks and public areas. Cabled ACB can have superior hydraulic stability to interlocking products, it is more difficult to remove (such as by vandalism), and it has improved constructibility when placed in water.



The table of ACB requirements is comprehensive, and many of the properties are interrelated. Including all listed properties will be specification overkill for most projects. Renumber notes where properties are deleted.

The critical shear stress is preferred by the industry over critical velocity because the critical shear is relatively constant for a product. Critical velocity varies depending on flow characteristics (such as depth and turbulence); so that the critical velocity in the flume test is not comparable to the required critical velocity in the field.

The surface void area ratio, DCF, and curvature radius can be used to specify products best suited to placement on curved surfaces, and turf establishment. Some blocks are available with open cells or solid cores. Most products marketed specifically for ACB revetments are within reasonable limits for general use.

Modifying the requirements for a performance specification may be considered for small low hazard projects or negotiated contracts, and may be preferred by some manufacturers. However, performance requirements for firm fixed price (low bid) contracts is not recommended for ACB's because of the price level jumps between product sizes, and because there is no control in how bidders incorporate risk for a low frequency design event.

Interface friction for soil/fabric and fabric/blocks should be considered by the designer. Because typical design values for soil/fabric are available in geosynthetic design guides, and because the Contractor generally has no control over the soil, the soil/fabric interface friction angle is not addressed in the specification (although for certain critical applications, this may need to be added to verify design assumptions). Interface friction for fabric/blocks is addressed in the specification because it is affected by geotextile and block combinations, it can affect installation, and because the interface friction for fabric/block is highly variable dependent on manufacturing characteristics of both products.

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The ACB shall meet the following criteria:

TABLE 1. ACB Requirements

<u>Criteria</u>	<u>Required Value</u>	<u>Test Method</u>
Matrix Assembly: Interlocking Blocks / Cabled System		
Thickness, minimum	[100 mm]	N/A

TABLE 1. ACB Requirements

Criteria	Required Value	Test Method
[Net Weight/Area, minimum	1.4 kN/m <sup>2</sup>	Note a.]
Critical Shear Stress, minimum	[170 N/m <sup>2</sup> ]	FHWA RD-89-199
[Critical Velocity, minimum	5 m/sec	FHWA RD-89-199]
[Curvature Radius, maximum	1 meter	Note b.]
[Surface Void Area Ratio	25% - 40%	Note c.]
[Drainage Correction Factor (DCF)	20% - 35%	Note d.]
Block/Geotextile Interface		
Friction Angle	[35 degrees]	Note e.

TABLE 1. ACB Requirements

Criteria	Required Value	Test Method
Matrix Assembly: Interlocking Blocks / Cabled System		
Thickness, minimum	[4 inches]	N/A
[Net Weight/Area, minimum	30 psf	Note a.]
Critical Shear Stress, minimum	[3.5 psf]	FHWA RD-89-199
[Critical Velocity, minimum	15 ft/sec	FHWA RD-89-199]
[Curvature Radius, maximum	3 feet	Note b.]
[Surface Void Area Ratio	25% - 40%	Note c.]
[Drainage Correction Factor (DCF)	20% - 35%	Note d.]
Block/Geotextile Interface		
Friction Angle	[35 degrees]	Note e.

a. The weight of the mattress per unit area shall be determined with the nominal joint spacing, in a non-submerged condition.

b. The curvature radius shall be indicative of the ability of the assembled mattress to conform to one dimensional subgrade curves without binding, such as for anchor trenches and swales. The curvature radius shall be demonstrated, if requested by the Contracting Officer.

c. The surface void area ratio shall be determined at the visible (with filled voids) surface of the blocks, with the joints spaced in a neutral position (50%), and shall be expressed as a percentage of the gross mat area. The void area shall include area between the blocks and open cells within the block.

d. The drainage correction factor shall be the minimum void area ratio (usually taken at the base of the blocks), with the joints spaced in a neutral position (50% freeplay in each direction), and shall be expressed as a percentage of the gross mat area.

e. The concrete surface shall be sufficiently rough to prevent sliding of the blocks on the geotextile. The interface friction must be matched with the selected block and geotextile combination, and shall be included with the ACB and Geotextile Data submittal. The block/geotextile interface friction angle shall be demonstrated, if requested by the Contracting Officer.

#### 2.1.1 Hydraulic Stability

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**NOTE: The velocity and shear stress conditions derived from this test are critical state conditions, and do not represent allowable design**

values. The surface tolerances for block placement are generally better in the test than field conditions.

FHWA RD-89-199 is a research document, not a standard test method. ASTM committee D18.25.04 has a draft standard based on a flume test, similar to FHWA RD-89-199. The flume test is very expensive: testing expenses can be on the order of \$30,000.

FHWA RD-89-199 included a 3/4" Enkamat fabric below the blocks for the purpose of installing instrumentation to research block behavior. The Enkamat provided a very effective drainage layer that is not integral to the test method. A drainage layer provides a very significant improvement in the ACB stability in the flume test. Regardless of the flume test conditions, the designer should consider including a granular drainage layer in areas with high turbulence flow.

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#### 2.1.1.1 Flume Test

The ACB product shall have been tested in a flume chamber in substantial conformance with FHWA RD-89-199, except that a drainage layer is not required. If the product was tested with a drainage layer, the installed product shall incorporate a similar drainage layer with adequate filtration design for the site soils. The flume test shall be based on conservative assumptions for field placement of the blocks (such as block orientation, and joint spacing within construction tolerances). The critical shear stress (and critical velocity) shall be indicated in the test report.

#### 2.1.1.2 Extrapolation of Hydraulic Stability

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NOTE: Preliminary research has indicated that extrapolation is conservative when extrapolating to thicker blocks, and unconservative when extrapolating to thinner blocks.

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Extrapolation of critical shear stress for untested blocks within a similar family of ACB shall be subject to limitations. Extrapolation shall only be used for blocks having a similar footprint area and interlock mechanism, but with variable thickness or net weight/area. Extrapolation shall only be accepted if the following conditions are met:

- a. The extrapolation is in strict accordance with hydraulic similitude methods commonly accepted by the industry, and includes quantitative treatment for a block overturning failure mode.
- b. The tested block is the smaller product size in both thickness and net weight/area, and extrapolation does not extend the critical velocity more than 3 meters per second 10 feet per second from the tested product size.

### 2.1.2 Matrix Assembly - Interlocking Blocks

Interlocking blocks are assumed to function without the use of cables or similar restraints. Void filler shall be placed to inhibit lateral movement and block pullout, cover the geotextile, and increase hydraulic stability.

### 2.1.3 Matrix Assembly - Cabled Systems

Cable tied concrete block shall be interconnected by flexible cables running through the blocks. Each block shall be penetrated by a cable that allows articulation of the blocks, but restrains removal of individual blocks. Void filler shall be placed to inhibit lateral movement, cover the geotextile, and increase hydraulic stability. [Articulating concrete block, cables, and fittings shall be fabricated into mattresses at the manufacturer's plant.]

### 2.1.4 Structural requirements

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NOTE: Freeze-thaw Testing - The specifier should edit this paragraph based on the project's location.

The default values correspond to the default values in ASTM C 1372, Segmental Concrete Retaining Wall Units. The freeze thaw requirements for retaining wall blocks are referenced since they are similar products, ACB blocks can be produced by similar manufacturing methods, and retaining walls have been the subject of more freeze thaw durability research.

The number of seasonal freeze thaw cycles for typical ACB's is likely less than for retaining walls. Chloride from deicing salts or sea water increases saturation of the concrete, and thus has a very pronounced affect on freeze thaw degradation.

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Articulating concrete block shall be wet cast using concrete as specified herein, or dry-cast by a vibratory block forming machine. The blocks shall be manufactured to the following requirements:

a. The minimum compressive strength shall be 28 MPa 4000 psi for an average of 3 units, and 24 MPa 3500 psi for an individual unit. Compressive strength shall be determined by ASTM C 42/C 42M for wet cast blocks, or by ASTM C 140 for dry cast blocks.

b. The maximum water absorption for dry cast units shall be 145 kg/m<sup>3</sup> 9 pcf for an average of 3 units, and 195 kg/m<sup>3</sup> 12 pcf for an individual unit. Water absorption shall be determined by ASTM C 140.

[c. The minimum saturated surface-dry density shall be [140] [\_\_\_\_\_] for average of 3 units, and [140] [\_\_\_\_\_] for an individual unit.]

[d. Wet cast concrete shall be air entrained to contain between 4 and 7 percent total air.]

[e. For freeze-thaw durability tested in accordance with ASTM C 1262, specimens shall comply with either of the following: (1) the weight loss of each of 5 specimens after 100 cycles shall not exceed 1 percent; or (2) the weight loss of each of 5 specimens after 150 cycles

shall not exceed 1.5 percent.]

## 2.2 GEOTEXTILE

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NOTE: The AASHTO M 288 table provides survivability requirements. Class 1 is recommended for harsh or severe installation conditions where there is a potential for vehicular traffic, or where irregular sections may require removal and replacement of mattresses to achieve proper alignment. Class 2 is allowed where no vehicle traffic will occur on the installation, and where mattress placement is in regular, even reaches. Reference Protection of Work paragraph in Part 3.

Some manufactures require minimum geotextile properties for warranty coverage. Most manufacturers have recommended geotextiles, but these should be verified for compatibility with the subgrade soils.

Filters should not impede seepage. Clogging resistance is critical for uplift stability. Site specific design should be performed if any of the following problematic soil conditions are encountered: highly erodible soils such as non-cohesive silts, gap graded soils, or laminated sand/silt.

Some references for geotextile design include:

1. Koerner, "Designing with Geosynthetics", Prentice Hall.
2. FHWA, "Geosynthetic Design and Construction Guidelines."
3. Geosynthetics '95, "Geotextile Permeability Criteria for Revetments", pp. 217-230.

Geosynthetic Selection - The Federal Acquisition Regulations require full and open competition. Usually justification is not necessary if 3 products meet the specifications. In combining various material requirements, it is easy to specify a geosynthetic product that does not exist. Design utilizing geosynthetics should include a listing with the calculations that verify the specified products are commercially available. The Geotechnical Fabrics Report magazine publishes an annual specifiers guide that is ideal for this purpose.

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Geotextile used as filters below the ACB shall be a [woven] [non-woven] fabric. The geotextile shall meet the material properties specified in AASHTO M 288 for Class [2] strength property requirements and for permanent erosion control. Filter requirements in AASHTO M 288 shall be based on in-situ soil with [less than 15%] [15% to 50%] [greater than 50%] passing the 0.075 mm sieve opening. SPECIFIC=GEOTEXTILE> SPECIFIC=GEOTEXTILE> SPECIFIC=GEOTEXTILE> SPECIFIC=GEOTEXTILE> 2 Geotextile used as a filter

below the ACB shall be a [woven] [non-woven] fabric, and shall meet the requirements specified in Table 2. The property values (except for AOS) represent minimum average roll values (MARV) in the weakest principal direction.

SPECIFIC=GEOTEXTILE> SPECIFIC=GEOTEXTILE> SPECIFIC=GEOTEXTILE>  
SPECIFIC=GEOTEXTILE> 2

TABLE 2. GEOTEXTILE PHYSICAL PROPERTIES

<u>PROPERTY</u>	<u>TEST REQUIREMENT</u>	<u>TEST METHOD</u>
Grab Tensile, N	[700 nonwoven] [1100 woven]	ASTM D 4632
Tear Strength, N	[250 nonwoven] [400 woven]	ASTM D 4533
Puncture Strength, N	[250 nonwoven] [400 woven]	ASTM D 4833
Permittivity, 1/sec	[0.5]	ASTM D 4491
Apparent Opening Size (µm)	[150 - 212]	ASTM D 4751
Ultraviolet Stability	[50%]	ASTM D 4355

TABLE 2. GEOTEXTILE PHYSICAL PROPERTIES

<u>PROPERTY</u>	<u>TEST REQUIREMENT</u>	<u>TEST METHOD</u>
Grab Tensile, lbs.	[160 nonwoven] [250 woven]	ASTM D 4632
Tear Strength, lbs.	[55 nonwoven] [90 woven]	ASTM D 4533
Puncture Strength, lbs.	[55 nonwoven] [90 woven]	ASTM D 4833
Permittivity, 1/sec	[0.5]	ASTM D 4491
Apparent Opening Size (U.S. Sieve)	[70 - 100]	ASTM D 4751
Ultraviolet Stability	[50%]	ASTM D 4355

## 2.3 CABLE

### 2.3.1 Installation Requirements for Cable

Cable used for preassembled mattresses shall be sufficiently sized and fastened for the size/weight of the assembled mattresses such that the assembled mattresses can be placed in compliance with OSHA standards. The manufacturer shall be responsible for determining the minimum cable strength compatible with the mattress size for safe handling. Cable strength shall be based on a minimum factor of safety of 5, and include appropriate reduction factors for mechanically crimped cable, and other fasteners. If applicable, loading conditions shall include the use of a

spreader bar for placing the mattresses.

#### 2.3.2 Fasteners Other than Cable

Any systems which rely on geotextiles (or other fabric integral with the mattress) to maintain block-to-block interconnection shall meet the applicable portions of this specification for cables. Geosynthetics strength shall include appropriate factors of safety, with particular attention given to the grab points.

#### 2.3.3 Design Requirements for Cable

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NOTE: The designer may need to research  
survivability of cables in the environment where the  
ACB will be placed. The installation requirements  
for cable strength usually govern, unless anchors  
are used.  
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ACB's that rely on cables to maintain block to block interconnection shall use ropes manufactured from polyester, stainless steel wire, or galvanized steel wire. The cable shall have a minimum breaking strength of [\_\_\_\_\_] pounds. Polyester rope shall be constructed of high tenacity, low elongating, continuous filament polyester fibers; and shall consist of a core construction comprised of parallel fibers contained within an outer jacket or cover.

#### 2.3.4 Anchors

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NOTE: Anchors require a cabled system. Anchors  
should be used with caution. Control of mattress  
uplift may only be successful for short term, low  
frequency, overload. ACB mattresses that are  
locally uplifted, with or without anchors, may show  
significant subgrade distortion, which could be a  
precursor to failure. Anchors spaced throughout the  
revetment also require special details to maintain  
integrity of the geotextile filter.

Because anchors are not required for typical  
installations, this paragraph may commonly be  
deleted. Where anchors are used for specialized  
applications, add requirements for anchor materials,  
minimum property requirements, and design  
characteristics.

Drawings must detail location and spacing of anchors.

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Anchors shall be selected with an ultimate vertical pullout resistance for the project site soil conditions of at least [\_\_\_\_\_] pounds. Anchors shall be capable of being attached directly to the articulating concrete block mat in a manner which will achieve little or no slack in the cable system or gaps in the articulating concrete block mattress. Anchors shall be attached to the mat in such a manner that they will not be affected by tampering or vandalism. Anchors shall have the capability of being load-tested to the specified pull-out capacity.

## 2.4 VOID FILLER

### 2.4.1 Aggregate

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NOTE: Aggregate is used below the normal water level, or where turf can not be established. Department of Transportation specifications for road base aggregate have been used, such as ASTM D 1241, Gradation B.  
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Aggregate for filling the voids in the block shall meet the requirements of [\_\_\_\_\_].

### 2.4.2 Topsoil and Seed

Topsoil for filling the voids in the block and seed for turf establishment shall meet the requirements of Section 32 92 19 SEEDING.

## PART 3 EXECUTION

### 3.1 SUBGRADE PREPARATION

The ACB revetment shall be placed on undisturbed native soils, or acceptably placed and compacted fill. The ACB shall not be placed on surfaces that contain mud, frost, organic soils, embankment that has not met compaction requirements, or where the Contracting Officer determines that unsatisfactory material remains in or under the subgrade.

#### 3.1.1 Clearing

All vegetation shall be completely removed as specified in Section [31 00 00 EARTHWORK] [31 11 00 CLEARING AND GRUBBING]. Remaining roots from trees and brush shall be removed to a depth of 0.3 meters 1 foot below the subgrade surface. Loose roots and twigs, turf clods, stones larger than 13 mm 1/2 inch diameter, and other debris shall be raked and removed from the final surface. Rills and gullies from erosion shall be corrected.

#### 3.1.2 Bank Grading

Grading shall be finished to a smooth surface, typical of that obtainable with a dozer and blade. A rough surface typically obtained with a backhoe or dragline will not be acceptable, except when ACB placement in water is shown on the drawings or approved by the Contracting Officer. [When natural shorelines require grading in preparation for ACB installation, the bank stratification shall be observed and documented in daily Contractor Quality Control reports. Grading practices shall avoid spreading fine grained soils over more pervious soils, particularly near the toe of slopes. If inadequate material is available to comply with this requirement, the Contracting Officer shall be notified.]

#### 3.1.3 Compaction and Subgrade Finishing

[Fill soils shall be compacted to the specified density in Section 31 00 00 EARTHWORK.] Incidental grading (where embankment is not otherwise specified) shall be compacted by heavy equipment or by tamping with a bucket to a density characteristic of the surrounding soils. The final



surfaces accessible by compaction equipment shall be compacted with a smooth drum roller or vibratory plate tamper until there is no further evidence of consolidation. Where slopes limit operation of compaction equipment, the final surface shall be back-dragged to a dense smooth surface with bladed equipment. Localized loose or soft zones shall be corrected.

#### 3.1.4 Grade Tolerances

The grading tolerance shall be within 50 mm 2 inches from the prescribed elevations, with no abrupt variations that would cause unacceptable projections of individual blocks.

#### 3.1.5 Subgrade Surface Tolerances

The subgrade shall be maintained in a smooth condition between installation of the geotextile and the blocks. Windrows, stones, clods of cohesive soil, and irregularities shall be raked smooth. Ruts, rills and gullies resulting from traffic, precipitation runoff, groundwater seepage, etc. shall be corrected prior to installation of blocks.

### 3.2 GEOTEXTILE INSTALLATION

#### 3.2.1 General

The geotextile shall be laid flat and smooth so that it is in direct contact with the subgrade. The geotextile shall be free of tension, folds, and wrinkles. The number of seams and overlaps shall be minimized by selective orientation of geotextile panels, within the limitations of maintaining a consistent pattern. Geotextile shall be placed immediately prior to block installation, if necessary to limit damage to the geotextile from equipment or repeated pedestrian traffic and limit disturbance of the subgrade from precipitation or runoff.

#### 3.2.2 Geotextile Seams

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NOTE: Sewn, welded or glued seams are desirable for shoreline protection (or where flow reversal occurs). Substitute the following for sewn seams:

Seams shall be continuously sewn at the locations shown on the drawings. The minimum distance from the geotextile edge to the stitch line nearest to that edge shall be 75 mm (3 inches). Seam strength shall meet the minimum requirements specified in AASHTO M288 for a class 2 geotextile. Quality assurance samples shall be taken at the request of the Contracting Officer. The thread at the end of each seam run shall be tied off to prevent unraveling. Seams shall be on the top side of the geotextile to allow inspection. Skipped stitches or discontinuities shall be sewn with an extra line of stitching with a minimum of 450 mm (18 inches) of overlap.

Pins/staples may not be desirable where filtration is critical.

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Seams shall be overlapped a minimum of 450 mm 18 inches. Seams on slopes and butt end seams shall be shingled so that runoff and channel flow passes over the fabric. Geotextile panels shall be secured before block placement by adequate sandbags, spare blocks, or pins/staples.

### 3.3 BLOCK INSTALLATION

All placement of blocks shall be in accordance with the manufacturer's recommendations and the Contractor's approved shop drawings.

#### 3.3.1 Placement of Pre-Assembled Mattresses

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NOTE: The spreader bar may be deleted where tolerances are a minor concern. The spreader bar may have associated cost when placing one end of the mattress in water, due to work in disconnecting the rigging on the wet end.  
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[Placement of pre-assembled mattresses shall be done with mattresses attached to a spreader bar to aid in lifting, aligning and placing the mattresses. ]The mattresses shall be placed directly into position, with a maximum space or gap between mattresses of 75 mm 3 inches in excess of the nominal joint spacing of blocks within the mattress. Mattresses out of alignment shall be lifted and reset. Mattresses shall not be pushed or pulled laterally after they are in contact with the geotextile. No overlapping of mats will be accepted and no blocks shall project vertically more than 25 mm 1 inch beyond the adjacent blocks. [As adjacent mats are placed, they shall be secured to each other by fastening the protruding horizontal and vertical cable connections and end cable loops together along each side of the mats.]

#### 3.3.2 Hand Placement of Interlocking Blocks

Hand placed blocks shall be spaced to maximize the ACB ability to articulate. The Contractor shall use adequate alignment control, such as string lines, to keep the block pattern in alignment and the joint spacing consistent and uniform. Initially, no more than two working block rows shall progress simultaneously in the direction of placement. Additional working rows may be added after experience shows that true lines are maintained. The starting position for ACB placement shall be a convenient location for control of the block pattern alignment. The Contracting Officer shall approve of the starting position for placement of the ACB.

##### 3.3.2.1 Target Joint Spacing

Interlocking blocks shall be installed with a uniform aperture in the interlocking connections. The target joint spacing shall be neutrally spaced with equal free-play for the joint to open and close.

##### 3.3.2.2 Correction of Joint Spacing

If the block pattern becomes skewed to an extent that blocks bind, joints close, or blocks stickup, then the placed ACB that is determined to be out of tolerance shall be removed and replaced. Where the nonconformance of the joint spacing is due to project features, such as warped slopes or anchor trenches, then cast-in-place concrete joints shall be field located

in concurrence with the Contracting Officer.

#### 3.3.2.3 Maintenance of Joint Spacing

If the block pattern becomes skewed to an extent that the joint freeplay is not acceptable to the Contracting Officer, then cast-in-place concrete joints shall be field located as directed by the Contracting Officer.

#### 3.3.2.4 Block Layout Pattern Dependent on Project Features

If the block pattern is shown to be maintained parallel and perpendicular to selected project features, such as the crest/toe of levee/channel slopes, then field location of cast-in-place concrete joints shall be implemented as needed, and as directed by the Contracting Officer.

#### 3.3.3 Tolerances

Maximum acceptable block projections (vertical offset from adjacent blocks) for "installation in the dry" shall not exceed 0.5 inches for interlocking blocks 25 mm 1.0 inch for cabled systems. Typical block projections shall be less than half the maximum projections.

#### 3.4 ANCHORS

Anchors shall be carefully positioned for attachment to the articulating concrete block. Rigid shafts shall align with the ACB cables. Flexible anchors (cables, etc.) shall be linear between the ACB fastener and the restraining device before tensioning. Penetrations in the geotextile to allow for penetration of the anchor shall be sealed [in accordance with the drawing details].

#### 3.5 CONCRETE JOINTS

##### 3.5.1 General Requirements

Use of cast in place concrete joints shall be minimized to the extent practicable. The Contracting Officer shall be informed of all concrete joints not shown on shop drawings prior to field placement. Joints that shall require concrete include:

- Joints between cable tied mattresses where the joint is 75 mm 3 inches wider than the nominal joint.
- Joints where block interlock is discontinuous.
- Abutments where the ACB meets headwalls, pipe penetrations, or sidewalks.
- Any areas where there are partial blocks (to avoid small blocks with reduced hydraulic stability).

Field placed concrete shall be proportioned for similar strength and durability properties as the ACB concrete[, and shall meet applicable portions of Section [03 31 00.00 10 CAST-IN-PLACE STRUCTURAL CONCRETE] [03 30 04 CONCRETE FOR MINOR STRUCTURES]]. All cable ties and anchoring shall be completed prior to placing concrete.

### 3.5.2 Abutments

The ACB shall abut pipe outlets, retaining walls, flood walls, head walls, sidewalks, and other abutments in a neat appearance. Unless a specific detail is indicated on the drawings, voids shall be filled with partial blocks and the gap shall be filled with cast-in-place concrete. The concrete shall be installed flush with the surface of the blocks, and shall be float finished.

### 3.6 VOID FILLER AND SEEDING

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NOTE: Topsoil will consolidate in the voids.  
Overfilling above the top of block is common to  
increase the topsoil fill level, but may increase  
sedimentation during turf establishment.

Broadcast seeding of ACB is difficult because it is  
difficult to rake the seed in. Hydroseeding of ACB  
is difficult because of channelization of runoff and  
susceptibility of the void filler to desiccation.  
In areas where establishment of quality turf is  
important, it has been successful to blend a rich  
mixture of seed (about 10 times the normal  
application rate, or about 0.5 to 1 kg/cubic m (1 to  
2 pounds per cubic yard) into the topsoil prior to  
placing. Watering during establishment is more  
critical than typical turf due to the heat  
absorption of the concrete blocks and capillary  
break of the geotextile.

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The voids of the articulating concrete block mats shall be filled with  
topsoil, except that voids below the normal water level shall be filled  
with aggregate void filler. All cable ties and anchoring shall be  
completed prior to filling voids. Seeding and maintenance shall be  
completed in accordance with Section 32 92 19 SEEDING.

### 3.7 PROTECTION OF WORK

Work shall be protected against damage from subsequent operations.  
Displaced or broken blocks shall be removed and replaced to conform to all  
requirements of this section. Damaged material shall not be incorporated.  
Equipment shall not be allowed on the ACB that could crack, cause abrasion,  
or otherwise damage the blocks. Vehicles shall not operate directly on  
geotextile, except that rubber tired vehicles may operate directly on short  
reaches of geotextile that meets or exceeds AASHTO M 288 survivability  
requirements for Class 1 geotextile, if there is no rutting, if the vehicle  
access is necessary to accomplish the work, and if the Contracting Officer  
observes the operation and approves. Vehicles shall not operate on the ACB  
until (during or after) placement of void filler. Vehicle traffic on the  
ACB shall be restricted to light weight rubber tired vehicles, and where  
intermittent access is necessary to accomplish the work. Routine haul  
routes shall not be established on the ACB. These allowances shall not  
waive the Contractor's obligation to maintain the installation until  
acceptance, and verify that vehicle access does not crack, or in any way  
damage, the ACB.

### 3.8 QUALITY CONTROL TESTING

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NOTE: The manufacturer should have completed similar testing for it's own QC. The following testing is suggested as an acceptance check on a schedule typical for QA. The suggested frequency is on the order of 0.5% of construction cost.

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The following testing shall be performed independent of the manufacturing process, by an agency other than the manufacturer. The ACB blocks shall be sampled and tested for compressive strength, water absorption and unit weight. The sample frequency shall be 3 specimens for each 2500 m<sup>2</sup> 3000 SY. Test methods shall be consistent with those specified in PART 2 PRODUCTS.

-- End of Section --