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 - 3.14.6 Inspection of Welding
 - 3.14.7 Structural Sections-in-Place

-- End of Section Table of Contents --

having an architectural finish on exposed-to-view surfaces when required.

Details of reinforcement indicating reinforcing-bar schedules; location and size of welded-wire fabric; and tenons for prestressed concrete indicating the final stressing force in kips, as required.

Details of connections indicating end bearing minimums and anchorage devices and other items embedded in the precast structural concrete sections.

Location and details of concrete floor topping, when required.

Details of openings including the size of steel framing members as required.

Details of precast concrete filler blocks, as required.

Details of hangers for suspended ceilings, ducts, piping, lighting fixtures, conduit, or other construction, as required.

Precast concrete floor-unit cells that will be used for electrical raceways, when required.

When both fire-resistance-rated construction and nonrated construction are required, the location of fire-resistance-rated construction.

Cast-in-place normal-weight concrete, including concrete floor topping, is specified in Section 03 30 53 MISCELLANEOUS CAST-IN-PLACE CONCRETE.

Precast conventionally reinforced concrete wall panels, solid-section type, are specified in Section 03 45 00 PRECAST ARCHITECTURAL CONCRETE.

Precast-concrete roof slabs placed over purlings or joists spaced not more than 8 feet on center are specified in Section 03 41 16 PRECAST CONCRETE SLABS (MAX. SPAN 8' - 0" O.C.).

Sealing joints in exposed-to-view surfaces of precast concrete slabs, such as at ceilings and walls, is specified in Section 07 92 00 JOINT SEALANTS.

Painting exposed-to-view surfaces of precast concrete units such as ceilings, is specified in Section 09 90 00 PAINTS AND COATINGS.

When cells of precast concrete cellular floor units will be used for electrical raceways, the inspection of cells to be used for electrical raceways, cutting the floor units for inserts, and electrical raceway fittings are specified in Section 26 05 00.00 40

COMMON WORK RESULTS FOR ELECTRICAL.

Fire-resistance-rated construction using precast structural concrete sections is described in Underwriters Laboratories, Inc., "Fire Resistance Ratings (BXUV)" included in UL Fire Resistance Directory and the "Fire-Resistance Ratings" contained in AIA CO-1. Fire-resistance-rated construction limits the types of precast structural concrete sections; the requirements for end restraint; the concrete materials and proportions of concrete mix for floor top fill; the requirements for grouting and sealing joints; and the type of roof insulation and roof covering.

Edit this guide specification for project specific requirements by adding, deleting, or revising text. For bracketed items, choose applicable items(s) or insert appropriate information.

Remove information and requirements not required in respective project, whether or not brackets are present.

Comments and suggestions on this guide specification are welcome and should be directed to the technical proponent of the specification. A listing of technical proponents, including their organization designation and telephone number, is on the Internet.

Recommended changes to a UFGS should be submitted as a Criteria Change Request (CCR).

PART 1 GENERAL

1.1 REFERENCES

NOTE: This paragraph is used to list the publications cited in the text of the guide specification. The publications are referred to in the text by basic designation only and listed in this paragraph by organization, designation, date, and title.

Use the Reference Wizard's Check Reference feature when you add a RID outside of the Section's Reference Article to automatically place the reference in the Reference Article. Also use the Reference Wizard's Check Reference feature to update the issue dates.

References not used in the text will automatically be deleted from this section of the project specification when you choose to reconcile references in the publish print process.

The publications listed below form a part of this specification to the extent referenced. The publications are referred to within the text by the basic designation only.

ACI INTERNATIONAL (ACI)

ACI 211.1	(1991; R 2009) Standard Practice for Selecting Proportions for Normal, Heavyweight, and Mass Concrete
ACI 318/318R	(2008; Errata 2008) Building Code Requirements for Structural Concrete and Commentary
ACI 318M	(2008; Errata 2008; Errata 2009) Metric Building Code Requirements for Structural Concrete and Commentary
ACI SP-66	(2004) ACI Detailing Manual
ACI/MCP-2	(2009) Manual of Concrete Practice Part 2 - ACI 224R-01 to ACI 313R-97
ACI/MCP-3	(2009) Manual of Concrete Practice Part 3 - ACI 315-99 to ACI 343R-95
ACI/MCP-4	(2009) Manual of Concrete Practice Part 4 - ACI 345R-05 to 355.2R-04

AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS
(AASHTO)

AASHTO M 200	(1973; R 2007) Standard Specification for Epoxy Protective Coatings
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AMERICAN HARDBOARD ASSOCIATION (AHA)

AHA A135.4	(2004) Basic Hardboard
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AMERICAN NATIONAL STANDARDS INSTITUTE (ANSI)

ANSI A48.1	(1986) Concrete Construction - Forms for One Way Concrete Joist Construction
ANSI A48.2	(1986) Concrete Construction - Forms for Two Way Concrete Joist Construction

AMERICAN WELDING SOCIETY (AWS)

AWS A5.1/A5.1M	(2004; Errata 2004) Carbon Steel Electrodes for Shielded Metal Arc Welding
AWS D1.1/D1.1M	(2008; Errata 2009) Structural Welding Code - Steel
AWS D1.4/D1.4M	(2005; Errata 2005) Structural Welding Code - Reinforcing Steel

ASTM INTERNATIONAL (ASTM)

ASTM A 153/A 153M	(2009) Standard Specification for Zinc Coating (Hot-Dip) on Iron and Steel Hardware
ASTM A 185/A 185M	(2007) Standard Specification for Steel Welded Wire Reinforcement, Plain, for Concrete
ASTM A 283/A 283M	(2003; R 2007) Standard Specification for Low and Intermediate Tensile Strength Carbon Steel Plates
ASTM A 322	(2007) Standard Specification Steel Bars, Alloy, Standard Grades
ASTM A 36/A 36M	(2008) Standard Specification for Carbon Structural Steel
ASTM A 370	(2009a ¹) Standard Test Methods and Definitions for Mechanical Testing of Steel Products
ASTM A 416/A 416M	(2006) Standard Specification for Steel Strand, Uncoated Seven-Wire for Prestressed Concrete
ASTM A 421/A 421M	(2005) Standard Specification for Uncoated Stress-Relieved Wire for Prestressed Concrete
ASTM A 615/A 615M	(2009b) Standard Specification for Deformed and Plain Carbon-Steel Bars for Concrete Reinforcement
ASTM A 675/A 675M	(2003; R 2009) Standard Specification for Steel Bars, Carbon, Hot-Wrought, Special Quality, Mechanical Properties
ASTM A 82/A 82M	(2007) Standard Specification for Steel Wire, Plain, for Concrete Reinforcement
ASTM C 109/C 109M	(2008) Standard Test Method for Compressive Strength of Hydraulic Cement Mortars (Using 2-in. or (50-mm) Cube Specimens)
ASTM C 1107/C 1107M	(2008) Standard Specification for Packaged Dry, Hydraulic-Cement Grout (Nonshrink)
ASTM C 114	(2009) Standard Test Methods for Chemical Analysis of Hydraulic Cement
ASTM C 115	(1996a; R 2003) Standard Test Method for Fineness of Portland Cement by the Turbidimeter
ASTM C 117	(2004) Standard Test Method for Materials

	Finer than 75-um (No. 200) Sieve in Mineral Aggregates by Washing
ASTM C 123	(2004) Standard Test Method for Lightweight Particles in Aggregate
ASTM C 126	(2009) Standard Specification for Ceramic Glazed Structural Clay Facing Tile, Facing Brick, and Solid Masonry Units
ASTM C 127	(2007) Standard Test Method for Density, Relative Density (Specific Gravity), and Absorption of Coarse Aggregate
ASTM C 128	(2007a) Standard Test Method for Density, Relative Density (Specific Gravity), and Absorption of Fine Aggregate
ASTM C 131	(2006) Standard Test Method for Resistance to Degradation of Small-Size Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine
ASTM C 136	(2006) Standard Test Method for Sieve Analysis of Fine and Coarse Aggregates
ASTM C 138/C 138M	(2009) Standard Test Method for Density ("Unit Weight"), Yield, and Air Content (Gravimetric) of Concrete
ASTM C 142	(1997; R 2004) Standard Test Method for Clay Lumps and Friable Particles in Aggregates
ASTM C 143/C 143M	(2009) Standard Test Method for Slump of Hydraulic-Cement Concrete
ASTM C 150	(2007) Standard Specification for Portland Cement
ASTM C 151/C 151M	(2009) Standard Test Method for Autoclave Expansion of Hydraulic Cement
ASTM C 157/C 157M	(2008) Standard Test Method for Length Change of Hardened Hydraulic-Cement Mortar and Concrete
ASTM C 172	(2008) Standard Practice for Sampling Freshly Mixed Concrete
ASTM C 173/C 173M	(2009) Standard Test Method for Air Content of Freshly Mixed Concrete by the Volumetric Method
ASTM C 183	(2008) Standard Practice for Sampling and the Amount of Testing of Hydraulic Cement
ASTM C 185	(2008) Standard Test Method for Air Content of Hydraulic Cement Mortar

ASTM C 191	(2008) Standard Test Method for Time of Setting Hydraulic Cement by Vicat Needle
ASTM C 192/C 192M	(2007) Standard Practice for Making and Curing Concrete Test Specimens in the Laboratory
ASTM C 204	(2007) Standard Test Method for Fineness of Hydraulic Cement by Air Permeability Apparatus
ASTM C 231	(2009a) Standard Test Method for Air Content of Freshly Mixed Concrete by the Pressure Method
ASTM C 232/C 232M	(2009) Standard Test Methods for Bleeding of Concrete
ASTM C 233	(2007) Standard Test Method for Air-Entraining Admixtures for Concrete
ASTM C 260	(2006) Standard Specification for Air-Entraining Admixtures for Concrete
ASTM C 266	(2008) Standard Test Method for Time of Setting of Hydraulic-Cement Paste by Gillmore Needles
ASTM C 29/C 29M	(2007) Standard Test Method for Bulk Density ("Unit Weight") and Voids in Aggregate
ASTM C 31/C 31M	(2009) Standard Practice for Making and Curing Concrete Test Specimens in the Field
ASTM C 33/C 33M	(2008) Standard Specification for Concrete Aggregates
ASTM C 330	(2005) Standard Specification for Lightweight Aggregates for Structural Concrete
ASTM C 39/C 39M	(2009) Standard Test Method for Compressive Strength of Cylindrical Concrete Specimens
ASTM C 40	(2004) Standard Test Method for Organic Impurities in Fine Aggregates for Concrete
ASTM C 403/C 403M	(2008) Standard Test Method for Time of Setting of Concrete Mixtures by Penetration Resistance
ASTM C 404	(2007) Standard Specification for Aggregates for Masonry Grout
ASTM C 42/C 42M	(2004) Standard Test Method for Obtaining and Testing Drilled Cores and Sawed Beams

of Concrete

ASTM C 451	(2008) Standard Test Method for Early Stiffening of Hydraulic Cement (Paste Method)
ASTM C 535	(2009) Standard Test Method for Resistance to Degradation of Large-Size Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine
ASTM C 566	(1997; R 2004) Standard Test Method for Total Evaporable Moisture Content of Aggregate by Drying
ASTM C 595	(2008a) Standard Specification for Blended Hydraulic Cements
ASTM C 618	(2008a) Standard Specification for Coal Fly Ash and Raw or Calcined Natural Pozzolan for Use in Concrete
ASTM C 70	(2006) Standard Test Method for Surface Moisture in Fine Aggregate
ASTM C 78	(2009) Standard Test Method for Flexural Strength of Concrete (Using Simple Beam with Third-Point Loading)
ASTM C 88	(2005) Standard Test Method for Soundness of Aggregates by Use of Sodium Sulfate or Magnesium Sulfate
ASTM C 94/C 94M	(2009a) Standard Specification for Ready-Mixed Concrete
ASTM C 989	(2009a) Standard Specification for Ground Granulated Blast-Furnace Slag for Use in Concrete and Mortars
ASTM D 1149	(2007) Standard Test Method for Rubber Deterioration - Surface Ozone Cracking in a Chamber
ASTM D 2103	(2008) Standard Specification for Polyethylene Film and Sheeting
ASTM D 2240	(2005) Standard Test Method for Rubber Property - Durometer Hardness
ASTM D 312	(2000; R 2006) Standard Specification for Asphalt Used in Roofing
ASTM D 3744	(2003) Standard Test Method for Aggregate Durability Index
ASTM D 395	(2003; R 2008) Standard Test Methods for Rubber Property - Compression Set

ASTM D 412	(2006aele2) Standard Test Methods for Vulcanized Rubber and Thermoplastic Elastomers - Tension
ASTM D 4397	(2009) Standard Specification for Polyethylene Sheeting for Construction, Industrial, and Agricultural Applications
ASTM D 471	(2006; R 2008) Standard Test Method for Rubber Property - Effect of Liquids
ASTM D 573	(2004) Standard Test Method for Rubber - Deterioration in an Air Oven
ASTM D 75/D 75M	(2009) Standard Practice for Sampling Aggregates
ASTM E 165	(2009) Standard Test Method for Liquid Penetrant Examination
ASTM E 648	(2009a) Standard Test Method for Critical Radiant Flux of Floor-Covering Systems Using a Radiant Heat Energy Source
ASTM E 709	(2008) Standard Guide for Magnetic Particle Examination

CONCRETE REINFORCING STEEL INSTITUTE (CRSI)

CRSI 10MSP	(2001; 27Ed) Manual of Standard Practice
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PRECAST/PRESTRESSED CONCRETE INSTITUTE (PCI)

PCI MNL-116	(1999) Manual for Quality Control for Plants and Production of Structural Precast Concrete Products
PCI MNL-120	(2004) Design Handbook - Precast and Prestressed Concrete

U.S. GENERAL SERVICES ADMINISTRATION (GSA)

FS MMM-A-001993	(1978) Adhesive, Epoxy, Flexible, Filled (For Binding, Sealing, and Grouting)
FS UU-B-790	(Rev A) Building Paper, Vegetable Fiber: (Kraft, Waterproofed, Water Repellent and Fire Resistant)

UNDERWRITERS LABORATORIES (UL)

UL Electrical Constructn	(2009) Electrical Construction Equipment Directory
UL Fire Resistance	(2009) Fire Resistance Directory

1.2 SUBMITTALS

NOTE: Review Submittal Description (SD) definitions in Section 01 33 00 SUBMITTAL PROCEDURES and edit the following list to reflect only the submittals required for the project. Submittals should be kept to the minimum required for adequate quality control.

A "G" following a submittal item indicates that the submittal requires Government approval. Some submittals are already marked with a "G". Only delete an existing "G" if the submittal item is not complex and can be reviewed through the Contractor's Quality Control system. Only add a "G" if the submittal is sufficiently important or complex in context of the project.

For submittals requiring Government approval on Army projects, a code of up to three characters within the submittal tags may be used following the "G" designation to indicate the approving authority. Codes for Army projects using the Resident Management System (RMS) are: "AE" for Architect-Engineer; "DO" for District Office (Engineering Division or other organization in the District Office); "AO" for Area Office; "RO" for Resident Office; and "PO" for Project Office. Codes following the "G" typically are not used for Navy, Air Force, and NASA projects.

Choose the first bracketed item for Navy, Air Force and NASA projects, or choose the second bracketed item for Army projects.

Government approval is required for submittals with a "G" designation; submittals not having a "G" designation are [for Contractor Quality Control approval.][for information only. When used, a designation following the "G" designation identifies the office that will review the submittal for the Government.] Submit the following in accordance with Section 01 33 00 SUBMITTAL PROCEDURES:

SD-02 Shop Drawings

Submit [Fabrication Drawings](#) in accordance with the paragraphs entitled, "Fabrication" and "Drawings," of this section.

Submit [Installation Drawings](#) in accordance with the paragraph entitled, "Drawings," of this section.

SD-05 Design Data

Submit mix design data in accordance with the paragraph entitled, "Concrete Design Mixes," of this section.

[Normal Weight Concrete](#)
[Lightweight Structural Concrete](#)

SD-06 Test Reports

Submit test reports for the following items in accordance with

paragraph entitled, "Concrete Sampling and Testing," of this section. Include within each report the project name and number, date, name of Contractor, name of precast-concrete manufacturer, name of concrete testing service, type of concrete, structural-member identification letter and number, design compressive strength at 28 calendar days, concrete-mix proportions and materials, compressive breaking strength and type of break, a record of gage pressures or dynamometer readings, compression strength of concrete at time of detensioning, and type of reinforcement. Design mix reports must be approved at least 15 calendar days prior to start of work.

Air Content

Air Entrainment

Compressive Strength

Slump

Moisture Content

Design Mix

Unit Weight

SD-07 Certificates

Include within the certificates of Compliance for the following items; qualifications of personnel, location of plant, concrete batching facilities, manufacturer equipment and facilities, a list of projects similar to specified work, handling and erection equipment, and performance requirements. Certificates for welder qualifications must be in accordance with the paragraph entitled, "[Qualifications for Welding Work](#)," of this section.

Installers

Manufacturer

Aggregate

Pretensioning

Detensioning

Submit [Welding Procedures](#) in accordance with [AWS D1.1/D1.1M](#).

SD-08 Manufacturer's Instructions

Indicate within the [Installation Instructions](#) the manufacturer's recommended sequence and methods of installation for the following items:

Welding Sequence and Procedure

Epoxy-Resin Grout

Epoxy-Resin Adhesive

1.3 QUALIFICATIONS FOR PRECAST-CONCRETE MANUFACTURER

Precast structural concrete sections must be manufactured by an organization experienced in the manufacture of precast concrete.

Submit a written description of the [Manufacturer](#) giving the qualifications of personnel, location of plant, concrete batching facilities, manufacturing equipment and facilities, list of projects similar to specified work, and other information as may be required.

Produce sections/units under plant-controlled conditions conforming to [PCI MNL-116](#) by a firm certified under the PCI Plant Certification Program and specializing in providing precast/prestressed products and related services.

1.4 QUALIFICATIONS FOR INSTALLER

Install members by an organization experienced in the installation of precast structural-concrete sections.

Submit a written description of [Installers](#) giving the qualifications of personnel, handling and erection equipment, list of projects similar to specified work, and other information as may be required.

1.5 QUALIFICATIONS FOR WELDING WORK

[Section [05 05 23 WELDING, STRUCTURAL](#) applies to work specified in this section.]

[[Welding Procedures](#) must be in accordance with [AWS D1.1/D1.1M](#).]

[Welders must be qualified by tests in accordance with [AWS D1.1/D1.1M](#).]

[Welders are to make only those types of weldments for which each is specifically qualified.]

Provide [installation instructions](#) for the [Welding Sequence and Procedure](#) which indicates the manufacturer's recommended sequence and method of installation.

1.6 PERFORMANCE REQUIREMENTS

1.6.1 Design Methods

Design must be in accordance with [ACI/MCP-3](#), [ACI 318/318R](#), [ACI 318M](#) and [PCI MNL-120](#).

1.6.2 Allowable Design Loads and Deflections

NOTE: Allowable design loads must be indicated and include dead loads, live loads, stationary loads, concentrated moving loads, deflection of roof slab sections, etc.

Recommended design loads are specified in article ix of the National Building Code, recommended by the American Insurance Association AIA CO-1 and ANSI

A58.1.

Allowable design loads and deflections must be as indicated.

1.6.3 UL Fire-Resistance Listing and Label

NOTE: Delete paragraph heading and the following paragraph when UL-listed fire-resistant precast structural concrete sections are not required. The UL lists several manufacturers of prestressed precast-concrete hollow-core flat slabs and single-tee and double-tee slabs. Location and fire-resistance classification of fire-resistant-rated structural sections must be indicated.

Sections indicated requiring a fire-resistance classification must be listed in **UL Fire Resistance**, part entitled, "Precast Concrete Units (CFTV)," and bear the UL label and marking.

1.6.4 Electrical Raceway UL Listing and Label

NOTE: Delete paragraph heading and the following paragraph when hollow-core floor-slab precast structural sections will not be used for electrical raceways, either under this contract or in the future. Location of electrical raceway structural sections must be indicated.

Hollow-core floor slabs indicated as electrical raceways must be listed in **UL Electrical Constructn**, part entitled, "Raceways (RGKT) Cellular Concrete Floor (RGYR)," and bear the UL label and marking.

1.7 CONCRETE SAMPLING AND TESTING

1.7.1 Tests for Concrete Materials

NOTE: Delete the following materials and tests that are not required.

Sample and test concrete materials proposed for use in the work as follows:

<u>MATERIAL</u>	<u>REQUIREMENT</u>	<u>TEST METHOD</u>	<u>NUMBER OF TESTS</u>
Concrete aggregates for normal-weight concrete	Sampling	ASTM D 75/D 75M	One for each material source and grading size
	Sieve analysis	ASTM C 136	
	Calculating fineness modulus	ASTM C 126	

<u>MATERIAL</u>	<u>REQUIREMENT</u>	<u>TEST METHOD</u>	<u>NUMBER OF TESTS</u>
	Amount of material passing 75 micrometer sieve	ASTM C 117	
	Amount of friable particles	ASTM C 142	
	Amount of organic impurities	ASTM C 40	
	Amount of coal and lignite	ASTM C 123	
	Magnesium sulfate soundness test	ASTM C 88	
	Aggregate durability	ASTM D 3744	
	Compact unit weight of slag (coarse aggregate)	ASTM C 29/C 29M	
	Resistance to abrasion test of small size coarse aggregate	ASTM C 131 or ASTM C 535	
Lightweight aggregates for structural concrete	Sampling	ASTM D 75/D 75M	One for each material source and grading size
	Sieve analysis	ASTM C 136 ASTM C 330	
	Compact unit weight (loose)	ASTM C 29/C 29M and ASTM C 330	
Lightweight structural concrete using the proposed lightweight aggregates	Specimen preparation	ASTM C 192/C 192M and ASTM C 330	As required for each type of test to determine conformance
	Compressive strength	ASTM C 39/C 39M	
	Unit-weight	ASTM C 330	
	Shrinkage	ASTM C 157/C 157M and ASTM C 330	

<u>MATERIAL</u>	<u>REQUIREMENT</u>	<u>TEST METHOD</u>	<u>NUMBER OF TESTS</u>
Hydraulic cement	Sampling	ASTM C 183	One for each material source, type, and color
	Chemical analysis	ASTM C 114	
	Fineness	ASTM C 115 or ASTM C 204	
	Autoclave expansion	ASTM C 151/C 151M	
	Time of setting	ASTM C 191 or ASTM C 266	
	Air content of mortar	ASTM C 185	
	Compressive strength	ASTM C 109/C 109M	
	Heat of hydration	ASTM C 185	
	False set	ASTM C 451	
Air entraining admixture using air-entraining concrete made of the proposed concrete materials	Materials for tests	ASTM C 233	One set of tests for each type of portland cement proposed for use and for each type of concrete
	Number of specimens	ASTM C 233, Table 1	
	Bleeding	ASTM C 232/C 232M	
	Time of setting	ASTM C 403/C 403M and ASTM C 233	
	Compressive-strength test specimen	ASTM C 192/C 192M and ASTM C 233	
	Compressive-strength test at 3, 7, and 28 calendar days	ASTM C 39/C 39M and ASTM C 233	
<u>MATERIAL</u>	<u>REQUIREMENT</u>	<u>TEST METHOD</u>	<u>NUMBER OF TESTS</u>
Concrete	Sampling	ASTM D 75/D 75M	One for each

<u>MATERIAL</u>	<u>REQUIREMENT</u>	<u>TEST METHOD</u>	<u>NUMBER OF TESTS</u>
aggregates for normal- weight concrete	Sieve analysis	ASTM C 136	material source and grading size
	Calculating fineness modulus	ASTM C 126	
	Amount of material pass- ing No. 200 sieve	ASTM C 117	
	Amount of fri- able particles	ASTM C 142	
	Amount of organic impurities	ASTM C 40	
	Amount of coal and lignite	ASTM C 123	
	Magnesium sul- fate soundness test	ASTM C 88	
	Aggregate dura- bility	ASTM D 3744	
	Compact unit weight of slag (coarse aggregate)	ASTM C 29/C 29M	
Lightweight aggregates for struc- tural con- crete	Resistance to abrasion test of small size coarse aggregate	ASTM C 131 or ASTM C 535	One for each material source and grading size
	Sampling	ASTM D 75/D 75M	
	Sieve analysis	ASTM C 136 ASTM C 330	
Lightweight structural concrete us- ing the pro- posed light- weight aggregates	Compact unit Unit weight (loose)	ASTM C 29/C 29M and ASTM C 330	As required for each type of test to deter- mine conformance
	Specimen preparation	ASTM C 192/C 192M and ASTM C 330	
	Compressive strength	ASTM C 39/C 39M	

<u>MATERIAL</u>	<u>REQUIREMENT</u>	<u>TEST METHOD</u>	<u>NUMBER OF TESTS</u>
Hydraulic cement	Unit-weight	ASTM C 330	One for each material source, type, and color
	Shrinkage	ASTM C 157/C 157M and ASTM C 330	
	Sampling	ASTM C 183	
	Chemical analysis	ASTM C 114	
	Fineness	ASTM C 115 or ASTM C 204	
	Autoclave expansion	ASTM C 151/C 151M	
	Time of setting	ASTM C 191 or ASTM C 266	
	Air content of mortar	ASTM C 185	
	Compressive strength	ASTM C 109/C 109M	
	Heat of hydration	ASTM C 185	
Air entrain- ing admix- ture using air-entrain- ing concrete made of the proposed concrete materials	False set	ASTM C 451	One set of tests for each type of port-land cement proposed for use and for each type of concrete
	Materials for tests	ASTM C 233	
	Number of specimens	ASTM C 233, Table 1	
	Bleeding	ASTM C 232/C 232M	
	Time of setting	ASTM C 403/C 403M and ASTM C 233	
	Compressive-strength test specimen	ASTM C 192/C 192M and ASTM C 233	
	Compressive-strength test at 3, 7, and 28 calendar days	ASTM C 39/C 39M and ASTM C 233	

<u>MATERIAL</u>	<u>REQUIREMENT</u>	<u>TEST METHOD</u>	<u>NUMBER OF TESTS</u>
1.7.2	Concrete Design Mixes		

NOTE: Delete the following types of concrete and tests not required.

Determine and test concrete **Design Mix** for concrete used as follows:

<u>TYPE OF CONCRETE</u>	<u>REQUIREMENT</u>	<u>TEST METHOD</u>	<u>NUMBER OF TESTS</u>
Normal weight concrete	Specific gravity and absorption of fine aggregate	ASTM C 128	As required for the concrete aggregates for each trial mix
	Specific gravity and absorption of coarse aggregate	ASTM C 127	
	Moisture Content tent of both fine and coarse aggregate	ASTM C 70 and ASTM C 566	
	Dry-rodded Unit Weight of coarse aggregate	ASTM C 29/C 29M	
	Trial mixes using at least three different water/cement ratios, minimum allowable cement content, maximum allowable slump; all with Air Entrainment	ACI 211.1	As required to determine the concrete mix having the properties specified in the paragraph entitled, "Quality of Concrete"
	Making and curing concrete specimens in the laboratory	ASTM C 192/C 192M	Two sets of three specimens for each design mix
	Sampling fresh concrete in the laboratory	ASTM C 192/C 192M	One for each set of design mix specimens
	Slump	ASTM C 143/C 143M	

<u>TYPE OF CONCRETE</u>	<u>REQUIREMENT</u>	<u>TEST METHOD</u>	<u>NUMBER OF TESTS</u>
	Air Content	ASTM C 231	
	Yield	ASTM C 138/C 138M	
	Compressive strength	ASTM C 39/C 39M	Three specimens tested at 28 calendar days
Lightweight structural concrete	Dry loose unit weight of aggregates	ASTM C 29/C 29M and ASTM C 330	As required for the lightweight aggregate for each trial mix
	Moisture content of aggregate	ASTM C 566	
	Trial mixes using at least three different water/cement ratios, maximum allowable slump; both with and without air entrainment	ACI 211.1	As required to determine the concrete mix having the properties specified in the paragraph entitled, "Quality of Concrete"
	Making and curing concrete the laboratory	ASTM C 192/C 192M	Two sets of for each design mix
	Sampling fresh concrete in the laboratory	ASTM C 192/C 192M	One for each set of design mix specimens
	Slump	ASTM C 143/C 143M	
	Air content	ASTM C 173/C 173M	
	Yield	ASTM C 138/C 138M	
	Compressive strength	ASTM C 39/C 39M	Three specimens tested at 7 calendar days and three specimens tested at 28 calendar days
	Air-dried unit weight	ASTM C 330	Two specimens tested after curing 28 calendar days

NOTE: Delete the following paragraph when

normal-weight concrete is not required.

From the results of the tests for normal-weight concrete, plot a curve showing the relationships between water/cement ratios and compressive strengths. Do not exceed the maximum water/cement ratio specified for normal-weight concrete properties shown by the curve to produce a design-minimum laboratory Compressive Strength at 28 calendar days not less than that specified.

**NOTE: Delete the following paragraph when
lightweight structural concrete is not required.**

From the results of the tests for lightweight structural concrete, plot a curve showing the relationships between cement contents and compressive strengths. Do not provide less than the minimum cement content specified for lightweight structural properties shown by the curve to produce a design-minimum laboratory compressive strength at 28 calendar days not less than that specified.

1.7.3 Quality Control Testing During Fabrication

**NOTE: Delete the following types of concrete not
required by the project.**

Sample and test concrete for quality control during fabrication as follows:

<u>TYPE OF CONCRETE</u>	<u>REQUIREMENT</u>	<u>TEST METHOD</u>	<u>NUMBER OF TESTS</u>
Normal weight concrete	Sampling of fresh concrete	ASTM C 172 except modified for slump per ASTM C 94/C 94M	As required for each test
	Slump test	ASTM C 143/C 143M	One for each concrete load at point of discharge and one for each set of compressive strength test
	Air Content by pressure method	ASTM C 231	One for each set of compres- sive-strength tests
	Compression test specimens	ASTM C 31/C 31M	One set of six standard cyl- inder speci- mens for each compressive

<u>TYPE OF CONCRETE</u>	<u>REQUIREMENT</u>	<u>TEST METHOD</u>	<u>NUMBER OF TESTS</u> strength test
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Curing of compression test specimens must be the same as the curing method used for the precast-concrete structural members.

	Concrete temperature		Each time a set of compression-test specimens is made
	Compressive strength tests	ASTM C 39/C 39M	One set for every ten structural members, or fraction thereof, cast in any one day; two specimens tested at 7 calendar days, three specimens tested at 28 calendar days, and one specimen retained in reserve for testing if required
Lightweight structural concrete	Sampling fresh concrete	ASTM C 172 except modified for slump per ASTM C 94/C 94M	As required for each test
	Slump test and unit weight of fresh concrete	ASTM C 143/C 143M ASTM C 138/C 138M	One for each concrete load at point of discharge and one for each set of compressive-strength tests
	Air content by volumetric method	ASTM C 173/C 173M	One for each set of compressive-strength tests
	Compressive test specimens	ASTM C 31/C 31M	One set of six standard cylinder specimens for each compressive-

<u>TYPE OF CONCRETE</u>	<u>REQUIREMENT</u>	<u>TEST METHOD</u>	<u>NUMBER OF TESTS</u> strength test
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The curing of Compressive Strength test specimens must be the same as the curing method used for the precast-concrete structural members.

	Concrete temperature		Each time a set of compression test specimens is made
	Compressive-strength tests	ASTM C 39/C 39M	One set for every ten structural members, or fraction thereof, as in any one day; two specimens tested at 7 calendar days, three specimens tested at 28 calendar days, and one specimen retained in reserve for testing if required
	Air-dried Unit Weight at 28 calendar days	ASTM C 330	One for each compressive strength test

Submit test results on the same day that tests are made.

1.8 DRAWINGS

Show type and location of all reinforcement, size and spacing of welds within [Fabrication Drawings](#).

Indicate type and location of all anchorage devices, size and spacing of all welded connections, grouting and joint sealant details, and dimensions and locations of all openings in structural concrete sections within [Installation Drawings](#).

PART 2 PRODUCTS

2.1 QUALITY OF CONCRETE

2.1.1 [Normal-Weight Concrete](#) Properties

NOTE: Delete paragraph heading and the following paragraphs when normal-weight concrete will not be

required.

<u>PROPERTY</u>	<u>VALUE</u>
Design Compressive Strength at 28 calendar days	Not less than 34.5 Megapascal
Maximum Aggregate size	19 millimeter
Maximum water/cement ratio	16 liter per 42.5 kilogram sack of cement
Slump at point of concrete discharge	Not to exceed 75 millimeter
Total air content by volume at point of concrete discharge	Not less than 4 percent nor more than 8 percent

<u>PROPERTY</u>	<u>VALUE</u>
Design Compressive Strength at 28 calendar days	Not less than 5,000 psi
Maximum Aggregate size	3/4 inch
Maximum water/cement ratio	4.25 gallons per 94-pound sack of cement
Slump at point of concrete discharge	Not to exceed 3 inches
Total air content by volume at point of concrete discharge	Not less than 4 percent nor more than 8 percent

2.1.1.2 Lightweight Structural Concrete Properties

**NOTE: Delete paragraph heading and the following
paragraphs when light-weight structural concrete
will not be required.**

<u>PROPERTY</u>	<u>VALUE</u>
Design compressive strength at 28 calendar days	Not less than 34.5 Megapascal
Maximum size Aggregate	19 millimeter
Minimum cement content	Seven 42.5 kilogram sacks of cement per 0.75 cubic meter
Slump at point of concrete	Not to exceed

<u>PROPERTY</u>	<u>VALUE</u>
discharge	75 millimeter
Total air content by volume at point of concrete discharge	Not less than 4 percent nor more than 8 percent
Air-dry density at 28 calendar days	Not less than 1440 nor more than 1840 kilogram per cubic meter

<u>PROPERTY</u>	<u>VALUE</u>
Design compressive strength at 28 calendar days	Not less than 5,000 psi
Maximum size Aggregate	3/4 inch
Minimum cement content	Seven 94-pound sacks of cement per cubic yard
Slump at point of concrete discharge	Not to exceed 3 inches
Total air content by volume at point of concrete discharge	Not less than 4 percent nor more than 8 percent
Air-dry density at 28 calendar days	Not less than 90 nor more than 115 pounds per cubic foot

2.2 CONCRETE MATERIALS

2.2.1 Aggregates

NOTE: Delete paragraph heading and the following paragraphs when precast structural-concrete sections will be fabricated of lightweight structural concrete. Precast concrete elements that will be exposed to the weather must be fabricated of normal-weight concrete. When an architectural finish, such as exposed aggregate, is required for exposed-to-view surfaces, refer to Section 03 45 00 PRECAST ARCHITECTURAL CONCRETE for concrete aggregate specifications.

Delete the following paragraph when both normal-weight concrete and lightweight structural concrete is required.

Aggregates must be fine and coarse conforming to ASTM C 33/C 33M and the following:

NOTE: Delete the following paragraph when precast structural concrete sections will be fabricated of normal-weight concrete.

Aggregates for normal-weight concrete must be fine and coarse conforming to **ASTM C 33/C 33M** and the following:

Where a structural member will be exposed to the weather meet the requirements of **ASTM C 33/C 33M** for fine aggregate subject to abrasion, for coarse aggregate subject to severe exposure, and for all concrete aggregates where surface appearance of the concrete is important.

Maximum size of coarse aggregate must be as specified.

2.2.2 Lightweight Aggregates

NOTE: Delete paragraph heading and the following paragraph when all precast structural-concrete sections will be fabricated of normal-weight concrete. Fire-resistance-rated structural sections may be fabricated of lightweight structural concrete, especially when the fire-resistance rating exceeds 2 hours.

Conform to **ASTM C 330** for fine and coarse aggregates in structural concrete.

2.2.3 Portland Cement

NOTE: If high early strength concrete is required, add Type III.

[Portland cement must conform to **ASTM C 150**, Type [____].]

[Blended hydraulic cement must conform to **ASTM C 595**, Type [____].]

Use one brand and type of cement for formed concrete having exposed-to-view finished surfaces.

2.2.4 Fly Ash

Fly ash [is required] [used] as an admixture [and] must conform to **ASTM C 618**, Class [C or F] with 4 percent maximum loss on ignition and between 15 to 35 percent maximum cement replacement by weight.

NOTE: Ground granulated blast furnace slag is one of the materials listed in the EPA's Comprehensive Procurement Guidelines (CPG) (<http://www.epa.gov/cpg/>). If the Architect/Engineer determines that use of certain materials meeting the CPG content standards and guidelines would result in inadequate competition,

do not meet quality/ performance specifications, are available at an unreasonable price or are not available within a reasonable time frame, the Architect/Engineer may submit written justification and supporting documentation for not procuring designated items containing recovered material. Written justification may be submitted on a Request for Waiver Form to the NASA Environmental Program Manager for approval. The Request for Waiver Form is located in the NASA Procedures and Guidelines (NPG 8830.1) (<http://nodis3.gsfc.nasa.gov>).

2.2.5 Ground Granulated Blast Furnace (GGBF) Slag

GGBF slag [is required] [used] as an admixture [and] must conform to [ASTM C 989](#), Grade [120] with between 25 to 50 percent maximum cement replacement by weight.

2.2.6 Air-Entraining Admixture

Admixture must be free of sodium chloride and nitrates and conform to [ASTM C 260](#).

2.2.7 Water

Water must be potable.

2.3 REINFORCEMENT MATERIALS

NOTE: Delete the following reinforcement materials that are not required. Concrete reinforcement materials are required for both conventionally reinforced and prestressed precast structural-concrete sections.

2.3.1 Reinforcement Bars

Bars must be deformed and conform to [ASTM A 615/A 615M](#), Grade 60, except that [9.5 millimeter diameter](#) bars may be Grade 40.

NOTE: Delete the following paragraph when galvanized reinforcing bars for concrete reinforcement will not be required. Galvanizing is recommended when the concrete cover over reinforcing bars is less than [38 millimeter 1-1/2 inches](#) for structural sections exposed to the weather.

Galvanize bars for structural sections exposed to the weather in accordance with [ASTM A 153/A 153M](#).

2.3.2 Cold-Drawn Steel Wire

Wire must conform to [ASTM A 82/A 82M](#).

2.3.3 Welded-Wire Fabric

NOTE: Select one of the following paragraphs as
applicable to the project.

Provide uncoated wire fabric conforming to [ASTM A 185/A 185M](#). Provide
galvanized wire fabric in structural sections exposed to the weather.

2.3.4 Supports for Concrete Reinforcement

Include bolsters, chairs, spacers, and other devices necessary for proper
spacing, supporting, and fastening reinforcement bars and wire in place.

Provide wire supports conforming to [ACI/MCP-4](#), [ANSI A48.1](#), [ANSI A48.2](#),
[ASTM E 648](#), [ACI SP-66](#) and [CRSI 10MSP](#).

Legs of supports in contact with formwork for sections that will be exposed
to weather must be hot-dip galvanized after fabrication, plastic coated, or
corrosion-resistant steel bar supports.

2.4 PRESTRESSING MATERIALS

NOTE: Delete paragraph heading and the following
paragraphs when prestressed structural-concrete
sections are not required.

2.4.1 Strand Tendons

NOTE: Strand tendons for prestressed concrete are
primarily intended for use in pretensioned, bonded,
prestressed concrete construction.

Provide uncoated, 7-strand, stress-relieved, steel wire conforming to
[ASTM A 416/A 416M](#).

2.4.2 Wire Tendons

NOTE: Delete paragraph heading and the following
paragraph when wire tendons for prestressed concrete
will not be required. Prestressing steel wire is
commonly used in prestressed linear concrete
construction in which the steel wire ends are
anchored by cold-end deformation (that is, button
anchorage) or in which the steel wire ends are
anchored by wedges.

Provide tendons conforming to [ASTM A 421/A 421M](#), Type BA or Type WA, as
required to suit the steel-wire anchorage method used.

2.4.3 Steel-Bar Tendons

NOTE: Delete paragraph heading and the following paragraphs when steel-bar tendons for prestressed concrete will not be required. Steel bars are principally used in post tensioning.

Provide uncoated round steel bars conforming to **ASTM A 322**.

Tensile properties of the bars after processing, when tested in accordance with **ASTM A 370**, must be as follows:

NOTE: Select one of the following values of tensile property and value as applicable to the project.

<u>TENSILE PROPERTY</u>	<u>VALUE NO. 1</u>	<u>VALUE NO. 2</u>
Ultimate tensile strength	1000 Megapascal min	1100 Megapascal min
Yield strength (0.2-percent offset)	900 Megapascal min	970 Megapascal min
Elongation at rupture in 20 diameters	4 percent min	4 percent min
Reduction on area at rupture	25 percent min	20 percent min
<u>TENSILE PROPERTY</u>	<u>VALUE NO. 1</u>	<u>VALUE NO. 2</u>
Ultimate tensile strength	145,000 psi min	160,000 psi min
Yield strength (0.2-percent offset)	130,000 psi min	140,000 psi min
Elongation at rupture in 20 diameters	4 percent min	4 percent min
Reduction on area at rupture	25 percent min	20 percent min

2.4.4 Tendon Anchorages for Pretensioning

Provide tendon anchorages capable of anchoring reinforcement without slippage after seating.

Steel cases for prestressing steel strand must be proof-tested by the manufacturer to at least 90 percent of the ultimate tensile strength of the strand.

2.4.5 Tendon Anchorages for Post Tensioning

NOTE: Delete paragraph heading and the following paragraphs when tendon anchorages for post tensioning will not be required. Normally, pretensioning only is required for prestressed precast structural concrete sections for building construction. Post tensioning may be required for field connections.

Anchorage must be capable of developing 100 percent of the guaranteed ultimate tensile strength of the reinforcement for prestressed concrete without excessive deformation. Provide anchorage plates of sufficient size to keep bearing pressures within the stress allowed by [ACI/MCP-3](#) and [ACI 318/318R](#) for the specified concrete strength at stressing.

Submit test data confirming the adequacy of anchorages.

2.5 CONNECTION MATERIALS

2.5.1 Steel Plates, Shapes, and Bars

Plates must conform to [ASTM A 283/A 283M](#), Grade C, or to [ASTM A 36/A 36M](#).

Structural-steel shapes must conform to [ASTM A 36/A 36M](#).

Bar shapes, flats, and rounds must conform to [ASTM A 675/A 675M](#), Grade 65, or [ASTM A 36/A 36M](#).

2.5.2 Steel Anchor Bolts

NOTE: Delete paragraph heading and the following paragraph when anchor bolts will not be required. Anchor bolts are normally required for precast concrete column base connections.

Anchor bolts must be steel with steel hexagon nuts and steel washers.

2.5.3 Electrodes for Welding

NOTE: Delete paragraph heading and the following paragraphs when welded connections will not be required.

Electrodes for manual shielded metal-arc welding connections consisting of structural quality carbon-steel members must conform to the AWS Code and be covered mild-steel electrodes conforming to [AWS A5.1/A5.1M](#), E60 series.

Electrodes for welding steel bars for concrete reinforcement must conform to [AWS D1.4/D1.4M](#).

2.5.4 Flexible Bearing Pads

NOTE: Delete one of the following paragraphs as applicable to the project. Delete paragraph heading

and the following paragraphs when flexible bearing pads are not required. Hardboard bearing pads are recommended for gravity connections having a bearing load not exceeding 1725 kilopascal 250 pounds per square inch (psi). Elastomeric nonlaminated bearing pads are recommended for gravity connections having a bearing load not exceeding 5500 kilopascal 800 psi. Where the bearing load exceeds 5500 kilopascal 800 psi or where there are small rotations, laminated type bearing pads designed and constructed to meet the requirements for loading and movement must be considered. The location and size of flexible bearing pads must be indicated.

Provide tempered hardboard pads not less than 3 millimeter 1/8 inch in thickness, smooth-two-sides, conforming to AHA A135.4.

Pads must be molded or cut from elastomeric material. Provide pad dimensions as indicated and within the following tolerances: thickness, plus or minus 1.5 millimeter 1/16 inch; width, minus 3 to plus 6.5 millimeter 1/8 to plus 1/4 inch; length, plus or minus 3 millimeter. 1/8 inch Material must be a vulcanized, chloroprene elastomeric compound conforming to the following tests:

<u>PROPERTY</u>	<u>TEST METHOD</u>	<u>PERFORMANCE</u>
Hardness Shore A durometer	ASTM D 2240	70 plus or minus 5 points
Tensile strength	ASTM D 412, Die C	Not less than 17.2 Megapascal
Ultimate elongation	ASTM D 412, Die C	Not less than 300 percent
Resistance to oil aging: change in volume after 70-hour immersion in ASTM oil No. 3 at 100 degrees C	ASTM D 471	Not more than plus 120 percent
Resistance to heat aging: change in original properties after 70 hours at 100 degrees C tensile strength ultimate elongation hardness	ASTM D 573	Plus 15 percent, minus 40 percent, 0 to plus 15 points
Resistance to permanent set: compression set after 22 hours at 100 degrees C	ASTM D 395 Method B	Not more than 35 percent
Resistance to ozone: condition after exposure of a sample kept under a surface tensile strain of 20 percent to an ozone concentration of 100 parts per million of air by	ASTM D 1149	No cracks

<u>PROPERTY</u>	<u>TEST METHOD</u>	<u>PERFORMANCE</u>
volume in air for 100 hours at 40 degrees C		

Not less than
91 kilogram per
25 linear millimeter

<u>PROPERTY</u>	<u>TEST METHOD</u>	<u>PERFORMANCE</u>
Hardness Shore A durometer	ASTM D 2240	70 plus or minus 5 points
Tensile strength	ASTM D 412, Die C	Not less than 2,500 psi
Ultimate elongation	ASTM D 412, Die C	Not less than 300 percent
Resistance to oil aging: change in volume after 70- hour immersion in ASTM oil No. 3 at 212 degrees F	ASTM D 471	Not more than plus 120 percent
Resistance to heat aging: change in original proper- ties after 70 hours at 212 degrees F tensile strength ultimate elongation hardness	ASTM D 573	Plus 15 percent, minus 40 percent, 0 to plus 15 points
Resistance to permanent set: compression set after 22 hours at 212 degrees F	ASTM D 395 Method B	Not more than 35 percent
Resistance to ozone: condition after exposure of a sample kept under a surface tensile strain of 20 percent to an ozone concentration of 100 parts per million of air by volume in air for 100 hours at 104 degrees F	ASTM D 1149	No cracks

Not less than
200 pounds per
linear inch

2.6 GROUTING MATERIALS

NOTE: Delete the following paragraphs that are not
applicable to the project. When fire-resistance
rated precast structural-concrete sections are
required, the applicable fire agency's requirements
for grouting materials must be consulted.

NOTE: Ground granulated blast furnace slag is one of the materials listed in the EPA's Comprehensive Procurement Guidelines (CPG) (<http://www.epa.gov/cpg/>). If the Architect/Engineer determines that use of certain materials meeting the CPG content standards and guidelines would result in inadequate competition, do not meet quality/ performance specifications, are available at an unreasonable price or are not available within a reasonable time frame, the Architect/Engineer may submit written justification and supporting documentation for not procuring designated items containing recovered material. Written justification may be submitted on a Request for Waiver Form to the NASA Environmental Program Manager for approval. The Request for Waiver Form is located in the NASA Procedures and Guidelines (NPG 8830.1) (<http://nodis3.gsfc.nasa.gov>).

Portland cement must conform to **ASTM C 150**, Type I.

Blended hydraulic cement must conform to **ASTM C 595**, Type [_____].

Aggregate for cement grout must conform to **ASTM C 404**, Size No. 2.

Shrinkage-resistant grouting compound must be premixed and packaged ferrous aggregate conforming to **ASTM C 1107/C 1107M**, for expansive grouts.

Water must be potable.

Provide two-component, mineral-filled, epoxy-polysulfide **epoxy-Resin Grout** conforming to **FS MMM-A-001993**, Type I.

Provide two-component, epoxy-polyamide cured type **epoxy-Resin Adhesive** conforming to **AASHTO M 200**.

2.7 BITUMINOUS JOINT SEALING MATERIALS

NOTE: Delete paragraph heading and the following paragraphs when single- or double-tee roof slab structural sections are not required.

Use asphalt bituminous cement conforming to **ASTM D 312**, Type IV.

Joint sealing tape must be **150 millimeter 6 inches** wide, multilayered, asphalt treated, glass-fiber reinforced, conforming to [**ASTM D 2103**] [**ASTM D 4397**] [**FS UU-B-790**, Type I, Grade C, Style 4,] with the following modification:

Dry tensile strength must not be less than **6130 newton per meter 35 pounds per inch** width, both directions.

2.8 FABRICATION

2.8.1 Fabrication Tolerances

**NOTE: Delete the following fabrication tolerances
that are not required by the project.**

Fabricate sections within the following tolerances:

Overall dimensions	Plus or minus 3 millimeter per 3048 millimeter but not greater than 19.1 millimeter overall
Cross-sectional dimensions of up to 150 millimeter	Plus or minus 3 millimeter
Over 150 to 460 millimeter	Plus or minus 4.8 millimeter
Over 460 to 915 millimeter	Plus or minus 6.4 millimeter
Over 915 millimeter	Plus or minus 9.5 millimeter
Deviation from straight line parallel to centerline of section up to 12.2 meter in length	Not over 9.5 millimeter
12.2 to 18.3 meter in length	Not over 12.7 millimeter
Over 18.3 meter in length	Not over 19.1 millimeter
Deviation from camber indicated on the drawings	Plus or minus 3 millimeter per 3 meter
Ends out of square, up to 305 millimeter in width or depth	0.80 millimeter per 25.4 millimeter of width or depth
Over 300 millimeter in width or depth	0.80 plus 0.40 millimeter per 25.4 millimeter of width or depth
Position of block-outs	Plus or minus 12.7 millimeter
Position of voids in hollow cored flat slabs, for both vertical and horizontal dimensions	Plus or minus 12.7 millimeter
Concrete cover over reinforcement	Plus 6.4, minus 0 millimeter
Position of tendons for pre- stressed concrete	Plus or minus 3.2 millimeter
Position of deflection points for deflected strand tendons	Plus or minus 152 millimeter

for prestressed concrete

Position of weld plates	Plus or minus 25.4 millimeter
Position of lateral anchorage points	Plus or minus 25.4 millimeter
Position of pickup devices	Plus or minus 152 millimeter
Overall dimensions	Plus or minus 1/8 inch per 10 feet but not greater than 3/4 inch overall
Cross-sectional dimensions of up to 6 inches	Plus or minus 1/8 inch
Over 6 to 18 inches	Plus or minus 3/16 inch
Over 18 to 36 inches	Plus or minus 1/4 inch
Over 36 inches	Plus or minus 3/8 inch
Deviation from straight line parallel to centerline of section up to 40 feet in length	Not over 3/8 inch
40 to 60 feet in length	Not over 1/2 inch
Over 60 feet in length	Not over 3/4 inch
Deviation from camber indicated on the drawings	Plus or minus 1/8 inch per 10 feet
Ends out of square, up to 12 inches in width or depth	1/32 inch per inch of width or depth
Over 12 inches in width or depth	1/32 inch plus 1/64 inch per inch of width or depth
Position of block-outs	Plus or minus 1/2 inch
Position of voids in hollow cored flat slabs, for both vertical and horizontal dimensions	Plus or minus 1/2 inch
Concrete cover over reinforcement	Plus 1/4, minus 0 inch
Position of tendons for pre-stressed concrete	Plus or minus 1/8 inch
Position of deflection points for deflected strand tendons for prestressed concrete	Plus or minus 6 inches
Position of weld plates	Plus or minus 1 inch

Position of lateral
anchorage points

Plus or minus 1 inch

Position of pickup devices

Plus or minus 6 inches

2.8.2 Forms

**NOTE: Structural-section dimensions,
cross-sections, and other details as required by the
project must be indicated.**

Use forms and form-facing materials that are nonreactive with concrete such as wood, metal, plastic, or other approved materials. Conform to the shapes, lines, and dimensions indicated and are within the limits of the specified fabrication tolerances.

2.8.3 Reinforcement

**NOTE: Reinforcement types, sizes, and arrangement
as required for structural strength after the
structural sections have been installed must be
indicated.**

Provide types, sizes, and arrangement as indicated on the approved drawings. Details of reinforcement must be in accordance with [ACI/MCP-3](#) and [ACI 318/318R](#), unless otherwise specified.

Place and secure steel bars, welded-wire fabric, and other reinforcement by means of metal bar supports and spacers.

**NOTE: Delete the following paragraph when
prestressed structural-concrete sections are not
required by the project.**

Place tendons and anchorages in accordance with [ACI/MCP-3](#) and [ACI 318/318R](#). End anchorages that will be permanently protected with concrete must be free of loose rust, grease, oil, paint, and other foreign matter. Bearing surface between anchorages and concrete must be perpendicular to and concentric with the tendons and the line of action prestressing force.

**NOTE: Revise the following paragraphs when not
applicable to the project. Concrete cover for
reinforcement must be indicated.**

Concrete cover for reinforcement must be in accordance with [ACI/MCP-3](#) and [ACI 318/318R](#).

2.8.4 Built-In Anchorage Devices

NOTE: Anchorage devices that are to be embedded in

the precast structural concrete sections must be indicated. Anchorage devices include weld plates, bearing plates and steel shapes.

Position, anchor, and locate anchorage devices where they do not affect the position of the main reinforcement or placing concrete. Bearing plates must be set level, aligned properly, and anchored in the exact location indicated.

2.8.5 Lifting Devices

Provide lifting devices designed for 100-percent impact, and of materials sufficiently ductile to ensure visible deformation before fracture.

2.8.6 Blockouts

NOTE: Blockouts or openings in slabs that would require the cutting of primary reinforcement if such openings were to be cut in the field must be cast in the unit during fabrication and must be indicated. The maximum size of field-cut openings may be from 150 to 300 millimeter 6 to 12 inches depending on the type of unit used such as the inside diameter of the voids in hollow cored flat slabs and the spacing of reinforcement.

Provide blockouts as indicated.

2.8.7 Pretensioning

NOTE: Delete paragraph heading and the following paragraphs when prestressed structural-concrete sections are not required by the project.

Pretensioning of tendons may be accomplished either by the single-strand or multiple-strand tensioning method. Determine the prestressing force by measuring the tendon elongation, either by checking the jack pressure on a recently calibrated gage or by use of a recently calibrated dynamometer. Correct any discrepancy that exceeds 5 percent. Base elongation requirements on the load-elongation curves for the type of tendon used. The total loss of prestress due to unreplaced broken tendons must not exceed 2 percent of the total prestress.

2.8.8 Concrete Mixing and Conveying

Measure concrete materials, concrete batching plant, concrete mixers, and concrete mixing in accordance with ASTM C 94/C 94M.

Handle concrete to prevent segregation and loss of concrete mix materials.

2.8.9 Preparations for Placing Concrete

Keep form interiors and reinforcement free of accumulations of hardened concrete, form-parting compound, standing water, ice, snow, or other

deleterious substances. Secure in position, inspect and approve reinforcement and other embedded items .

2.8.10 Weather Limitations

Do not place concrete when temperature of the atmosphere is below 5 degrees C 40 degrees F nor during rain, sleet, and snow unless adequate protection is provided. Protection during inclement weather must prevent the entry of rain, sleet, or snow into the forms or into the fresh concrete.

2.8.11 Concrete Placing

Depost concrete so that no concrete will be placed on concrete that has hardened sufficiently to cause formation of seams or planes of weakness. Consolidate concrete in a manner that will prevent segregation and will produce concrete free of honeycomb or rock pockets and with the required surface finish.

2.8.12 Identification Markings

Clearly mark each structural section in a permanent manner to indicate its location and orientation in the building and the pickup points.

Each structural section must have the date of casting plainly indented in the unexposed face of the concrete.

2.8.13 Finishing Unformed Surfaces

Trowel finish unformed surfaces unless otherwise specified. Provide smooth surface free of trowel marks, uniform in texture and appearance, and be plane to a tolerance not exceeding 3.2 millimeter in 3048 millimeter 1/8 inch in 10 feet when tested with a 3000 millimeter 10-foot straightedge.

Provide top surfaces of sections that are to receive concrete topping after installation with a transversely scarified scratch finish and remove laitance.

2.8.14 Curing

Cure concrete by keeping the concrete damp for not less than 7 calendar ays if made of Type I portland cement and for not less than 3 calendar days if made of Type III portland cement. For each decrease of 3 degrees below 21 degrees C 5 degrees below 70 degrees F in the average curing temperature, increase the curing period by 4 calendar days for concrete made of Type I portland cement and by 2 calendar days for concrete made of Type III portland cement.

Curing by low-pressure steam, steam vapor, radiant heat and moisture, or other acceptable process may be employed provided that the compressive strength of the concrete is equal to that obtained by moist curing and the 28-day compressive strength of the concrete meets the requirements specified, as determined by test cylinders of the same concrete cured by the same curing process.

Do not remove sections from their casting beds until the curing period is completed or concrete has attained at least 75 percent of its design compressive strength.

2.8.15 Protection of Concrete After Placing

Protection must meet the requirements of ACI/MCP-2 for hot or cold weather, as applicable.

2.8.16 Detensioning

NOTE: Delete paragraph heading and the following paragraphs when prestressed structural-concrete sections are not required by the project.

Detensioning of tendons must not be done until the concrete compressive strength, as indicated by test cylinders, is as follows:

<u>TYPE OF REINFORCEMENT</u>	<u>TRANSFER STRENGTH OF CONCRETE</u>
Concentrically stressed sections	Not less than 20 Megapascal
Eccentrically stressed sections	Not less than 24.1 Megapascal
Beams or other sections in which camber must be minimized	Not less than 27.6 Megapascal

<u>TYPE OF REINFORCEMENT</u>	<u>TRANSFER STRENGTH OF CONCRETE</u>
Concentrically stressed sections	Not less than 3,000 psi
Eccentrically stressed sections	Not less than 3,500 psi
Beams or other sections in which camber must be minimized	Not less than 4,000 psi

Remove test cylinders to be used to establish the compressive strength of the concrete from the casting bed at least 1 hour prior to the start of the detensioning operation. Allow test cylinders from heat-cured casting beds to cool for approximately 1/2 hour prior to capping, and allow caps of sulfur compound to cure for 1/2 hour prior to the compressive-strength test.

If concrete has been heat cured, the detensioning operation must be done following the curing period while the concrete is still warm and moist to avoid cracking or undesirable stresses in the concrete.

Prior to detensioning operations, forms, ties, inserts, holddowns, or other devices that would restrict the longitudinal movement of the sections along the casting bed must be removed or loosened to provide free movement of the structural section. Alternately, perform detensioning so that longitudinal movement is precluded.

In detensioning operations, prestressing forces must be kept nearly symmetrical about the vertical axis of the section and be applied in a manner that will minimize sudden or shock loading. Limit maximum

eccentricity about the vertical axis to one strand. Detensioning of pretensioned tendons may be accomplished either by gradual release of the tensioning jacks or by heat-cutting the tendons in accordance with an approved pattern and sequence to prevent severe unbalancing of the loading.

2.8.17 Finishing Formed Surfaces

Upon removal of forms, repair and patch defective areas. Limit defective areas to holes left by tie rods and other temporary inserts and to honeycomb or rock pockets not deep enough to expose the reinforcement and not located in bearing areas. Cut out defective areas to solid concrete and cleaned. Patches on lower side of sections, near the center or in areas of variable tensile strength, must be bonded by a two-component epoxy-polysulfide or epoxy-polyamine bonding adhesive. Other areas will be dampened with water and patched with portland cement grout. Where the concrete surface will be exposed to view, the patches, when dry, must match the surrounding concrete.

Formed surfaces of sections that will be concealed by other construction can have the standard smooth finish having the texture imparted by the forms. Repair and patch defective areas as specified and all fins and other projections removed.

NOTE: Delete the following paragraph and specify the required finish when an architectural finish is required. For an exposed-aggregate finish refer to Section 03 45 00 PRECAST ARCHITECTURAL CONCRETE. The location of precast structural concrete sections having an architectural finish must be indicated.

Provide grout finish on formed surfaces of sections that are to be exposed-to-view after installation. Final color of the grout, when dry, must be the same for all concrete surfaces. Spread over dampened concrete surface with clean burlap pads, carpet, or sponge rubber floats to fill pits, air bubbles, and surface holes. Remove excess grout by scraping and then rubbing the surface with clean burlap or carpet to remove visible grout film. In hot dry weather, kept grout damp by means of fog-spraying during the setting period.

PART 3 EXECUTION

3.1 GENERAL

Install sections in accordance with the approved drawings and as specified.

3.2 ANCHORAGE ITEMS EMBEDDED IN OTHER CONSTRUCTION

NOTE: Delete the paragraph heading and the following paragraph when precast structural-concrete sections will not be connected to cast-in-place concrete construction or masonry construction. Such anchorage items include anchor bolts, steel dowels, and steel bearing plates.

Deliver items to the site before the start of other construction. Provide

setting drawings, templates, instructions, and directions for the installation of anchorage items.

3.3 INSTALLATION OF FLEXIBLE BEARING PADS

NOTE: Delete paragraph heading and the following paragraphs when flexible bearing pads are not required. Bearing pads must be indicated.

Install pads where indicated, set in correct position, and have a uniform bearing. Keep in the correct position while placing sections.

3.4 STRENGTH OF STRUCTURAL SECTIONS AT INSTALLATION

NOTE: Delete one of the following paragraphs as applicable to the project. Select the first paragraph except when the project schedule indicates installation of 28-day structural sections.

Do not install sections until concrete has attained the specified minimum laboratory strength at 28 calendar days.

Do not install sections before 28 calendar days from the date of casting has elapsed unless approval has been obtained to make one compressive-strength test, [ASTM C 39/C 39M](#), and one flexural strength test using simple beam with third point loading, [ASTM C 78](#), on field cured concrete test specimens, [ASTM C 31/C 31M](#), for each individual structural section to determine the strength of the concrete.

3.5 INSTALLATION TOLERANCES

Install sections within the following tolerances:

Deviation in location from indicated	Plus or minus 6.4 millimeter
Deviation from plumb for columns in any story or 6.1 meter maximum	Not over 6.4 millimeter
In 12.2 meter or more	Not over 12.7 millimeter
Deviation from elevations indicated for girders, beams, joists, and slabs in any bay or 6.1 meter maximum	Not over 6.4 millimeter
In 12.2 meter or more	Not over 12.7 millimeter
Difference between adjacent structural sections in erected position	Plus or minus 1.6 millimeter per 3000 millimeter but not greater than 6.4 millimeter overall
Deviation in location from	Plus or minus 1/4 inch

indicated

Deviation from plumb for columns in any story or 20 feet maximum	Not over 1/4 inch
In 40 feet or more	Not over 1/2 inch
Deviation from elevations indicated for girders, beams, joists, and slabs in any bay or 20 feet maximum	Not over 1/4 inch
In 40 feet or more	Not over 1/2 inch
Difference between adjacent structural sections in erected position	Plus or minus 1/16 inch per 10 feet but not greater than 1/4 inch overall

3.6 PLACING FRAMING STRUCTURAL SECTIONS

NOTE: Delete paragraph heading and the following paragraphs when framing structural sections such as columns, beams, girders, and joists will not be required.

Place supporting sections, including anchorage items attached to or embedded in other construction before placing sections is started.

NOTE: Delete the following paragraphs when precast concrete columns with attached steel bearing plates will not be required.

Installation of precast concrete columns with attached steel bearing plates must be as follows:

Concrete and steel plate bearing surfaces must be cleaned of laitance, dirt, oil, grease, and other foreign materials. Roughen concrete surface.

Space between the top of the concrete bearing surface and the bottom of the steel plate must be approximately 1/24 of the width of the bearing plate, but not less than 12.7 millimeter 1/2 inch for bearing plate that is less than 300 millimeter 12 inches wide. Support and align bearing plate on steel wedges or shims.

After precast concrete columns have been positioned and braced and anchor bolts tightened, the space between the top of the bearing surface and the bottom of the steel bearing plate must be grouted.

Do not remove wedges or shims shall not be removed but, when protruding, cut off flush with the edge of the steel bearing plate prior to grouting.

Install sections plumb, level, and in alignment within the limits of the

installation tolerances specified.

3.7 PLACING SLAB STRUCTURAL SECTIONS

NOTE: Delete the paragraph heading and the following paragraphs when slab structural sections, such as single- and double-tee slabs and hollow-cored flat slabs will not be required. Slab structural-sections may be placed over structural-steel framing members, precast structural-concrete framing sections, cast-in-place structural-concrete framing sections, or bearing walls, or a combination thereof.

Supporting sections, including bearing pads or plates, must be in place before placing sections is started. Slab structural sections must be placed on supporting construction with ends bearing on the structural framing sections or bearing walls as indicated. End bearings must not be less than 75 millimeter 3 inches. Accurately align slabs end to end with sides and ends butted together. Provide grouting void at sides and ends of the slabs as indicated.

NOTE: Delete the following paragraph when electrical-raceway hollow-cored flat-slab structural sections will not be required.

Place electrical raceway hollow-cored flat-slab structural sections in straight alignment for the entire length of run of the hollow cores and with close alignment between hollow cores at the ends of abutting slab structural sections.

3.8 WELDED CONNECTIONS

NOTE: Welded connections are the most commonly used type of connection. Other types of connections that may be employed are gravity, structural-steel bolted, post-tensioned, cast-in-place reinforced-concrete, and doweled connections. Connection details must be indicated.

Welding reinforcing steel, metal inserts, and connections in precast-concrete structural-member construction must be in accordance with AWS D1.4/D1.4M.

Welding structural steel connections must be in accordance with AWS D1.1/D1.1M Code.

3.9 GROUTING CONNECTIONS AND JOINTS

NOTE: Delete paragraph heading and the following paragraphs when precast structural-concrete framing sections or floor-slab structural sections or both

will not be required. When fire-resistance-rated precast structural-concrete sections are required, consult the applicable fire agency's requirements for grouting joints.

After sections have been placed and connected, grout open spaces at connections and joints.

NOTE: Delete the following paragraph when shrink-resistant grout only is required.

Cement grout must be of 1 part cement, 2-1/2 parts of specified aggregate for cement grout, and not more than 17 liter 4-1/2 gallons of water per 42.6 kilogram 94-pound sack of cement.

NOTE: Delete the following paragraph when cement grout only is required.

Mix shrink-resistant grout compound with water to provide a flowable mixture without segregation or bleeding.

Provide forms or other approved methods to retain the grout in place. Pack spaces with grout until the voids are completely filled. Flush grout at slab structural sections with top surface of the slab and remove excess. Keep grout damp for not less than 24 hours.

NOTE: Delete the following paragraphs when cement grout only is required or when epoxy-resin grout or adhesive instead of shrink-resistant grout is not required.

Epoxy-resin grout or adhesive may be used in lieu of shrink-resistant grout. Installation of epoxy-resin grout or adhesive must be in accordance with the manufacturer's printed instructions.

NOTE: Delete the following paragraph when electrical raceway hollow-cored flat-slab structural sections are not required.

Open spaces at abutting ends of electrical raceway hollow-cored flat-slab structural sections must be sealed with pressure-sensitive tape. Hollow cores used for electrical raceways must be kept free from grout and other foreign materials.

3.10 SEALING JOINTS IN ROOF SLABS

NOTE: Delete paragraph heading and the following paragraphs when roof slab structural sections will not be required. Where fire-resistance-rated roof

slab structural sections are required, the applicable fire agency's requirements for sealing joints must be consulted.

After precast-concrete roof slab sections have been placed and connected, seal open spaces at connections and the top portion of joints.

Fill keyways and joints at ridges, hips, and connections with cement grout. Level with the top surfaces of slabs, remove excess grout, and apply a smooth finish.

Seal other joints with bituminous joint-sealing material. Center joint-sealing tape over the joint and embedded in hot bituminous cement. Lap Ends not less than 100 millimeter 4 inches. Remove excess bitumen and provide a smooth tape surface.

3.11 OPENINGS IN SLAB STRUCTURAL SECTIONS

NOTE: The maximum size of field-cut openings is governed by the spacing of reinforcement and the inside diameter of the voids in hollow-cored flat slabs.

Cut and fit sections as required for other work projecting through, or adjacent to, the members. Cuts must be straight and at 90 degrees to the surfaces without breaking or spalling the edges.

NOTE: Delete the following paragraph when hollow-cored flat-slab structural sections will not be required. Openings larger than the width of a slab structural section must be framed with supporting members.

Openings in hollow-core flat-slab sections having any dimension more than the inside diameter of the hollow cores and not exceeding the width of the slab structural section must be reinforced by means of hung steel angle saddle headers. Headers must be shop prime-coat painted and as indicated on the approved drawings.

3.12 TOUCHUP PAINTING

NOTE: Delete paragraph heading and the following paragraph when precast structural-concrete sections will not be supported by steel structural members.

After sections have been installed, scarred surfaces on steel supporting members and weld plates must be wire brushed, cleaned, and touchup painted.

3.13 PROTECTION AND CLEANING

NOTE: Where architectural finishes such as

exposed-aggregate finish are specified for
exposed-to-view surfaces, such surfaces must be
cleaned as specified in Section 03 45 00 PRECAST
ARCHITECTURAL CONCRETE.

Protect exposed-to-view surfaces against staining and other damage until
completion of the work.

Upon completion of installation, swept clean and leave ready slab surfaces
to receive concrete floor topping, roofing, or other covering.

3.14 INSPECTION AND ACCEPTANCE PROVISIONS

3.14.1 Evaluation of Compressive Strength Tests

Concrete quality control tests will be evaluated as specified.

**NOTE: Delete the following paragraph when
normal-weight concrete will not be required.**

Normal-weight concrete delivered to the point of placement having a slump
or total air content outside the values specified must not be used in the
work.

**NOTE: Delete the following paragraph when
lightweight structural concrete will not be required.**

Lightweight structural concrete delivered to the point of placement having
a unit weight of fresh concrete that varies more than 2 percent from the
design mix wet unit weight or having a slump or total air content outside
the values specified must not be used in the work.

Compressive-strength tests will be considered satisfactory if the average
of any group of 5 consecutive compressive-strength tests that may be
selected is in each instance equal to or greater than the 28-day design
compressive strength or if not more than one compressive-strength test in
10 has a value less than 90 percent of the 28-day design
compressive-strength.

If the compressive-strength tests fail to meet the minimum requirements
specified, the sections fabricated of concrete represented by such tests
will be considered deficient in strength and subject to the provisions
specified.

3.14.2 Dimensional Tolerances

Members having any dimension outside the limits for fabrication tolerances
specified will be rejected.

3.14.3 Surface-Finish Requirements

Sections will be rejected for any of the following surface-finish
deficiencies:

NOTE: Delete the first of the following paragraphs
when architectural finishes such as
exposed-aggregate finish, are not required for
exposed-to-view surfaces.

Exposed-to-view surfaces having architectural finishes that do not match the color, aggregate size and distribution, and texture of the approved sample for the exposed-to-view finish

Exposed-to-view formed surfaces that contain cracks, spalls, air bubbles, honeycomb, rock pockets, or stains or other discoloration that cannot be removed by cleaning

Concealed formed surfaces that contain cracks in excess of 0.25 millimeter 0.01 inch wide; cracks or any other surface deficiency that penetrates to the reinforcement regardless of the width of crack or size of other deficiency; honeycomb and rock pockets located in bearing surfaces; and spalls except minor breakage at corners

Unformed surfaces that contain cracks and other surface deficiencies as specified for concealed formed surfaces

3.14.4 Strength of Structural Members

Strength of precast structural-concrete sections will be considered potentially deficient if they fail to comply with the requirements that control the strength of the structural members, including the following conditions:

Failure to meet compressive strength tests

Reinforcement and pretensioning and detensioning of tendons of prestressed concrete not conforming to the requirements specified

Concrete curing and protection of structural sections against extremes in temperature during curing not conforming to the requirements specified

Structural sections damaged during handling and erection

3.14.5 Testing Structural Sections for Strength

When there is evidence that the strength of precast structural-concrete sections does not meet specification requirements, cores drilled in hardened concrete for compressive strength determination must be made in accordance with ASTM C 42/C 42M and as follows:

Take at least three representative cores from the precast structural concrete sections that are considered potentially deficient.

Test cores saturated-surface-dry if the concrete they represent will be wet at all times during the use of the completed structure.

Test cores air-dry if the concrete they represent will be dry at all times during the use of the completed structure.

Strength of cores will be considered satisfactory if their average is

equal to or greater than the 28-day design compressive strength of 150 by 300 millimeter 6-by 12-inch cylinders.

Fill core holes solidly with patching mortar and finished to match the adjacent concrete surfaces.

If the results of the core tests are unsatisfactory or if core tests are impractical to obtain, static load tests must be made of a structural section and will be evaluated in accordance with ACI/MCP-3 and ACI 318/318R, except that the superimposed test load must be as specified for the proof-test method of strength design.

Sections that are found inadequate by the core tests or by the results of static load tests must be replaced with sections that meet the specified requirements.

3.14.6 Inspection of Welding

NOTE: Delete paragraph heading and the following paragraphs when inspection of welding will not be required.

Perform inspection of welding in accordance with AWS D1.1/D1.1M, Section entitled, "Inspection," and as follows:

NOTE: Delete the following paragraphs that are not applicable to the project. The location of welds requiring inspection and the type of inspection must be indicated. The liquid-penetration inspection of welds is the most economical and commonly used method.

Liquid-penetration inspection of welds must conform to ASTM E 165.

Magnetic-particle inspection of welds must conform to ASTM E 709.

3.14.7 Structural Sections-in-Place

Sections-in-place will be rejected for any one of the following deficiencies:

Sections not conforming to the requirements for installation tolerances specified

Sections that are damaged during construction operations

Sections having exposed-to-view surface finishes that develop surface finish deficiencies specified

-- End of Section --